

## **A46 Newark Bypass**

**Scheme Number: TR010065**

### **7.40 Hydraulic Modelling Technical Note**

APFP Regulation 5(2)(q)

Planning Act 2008

Infrastructure Planning (Examination Procedure) Rules  
2010

February 2025

Volume 7

Infrastructure Planning

Planning Act 2008

**The Infrastructure (Examination  
Procedure) Rules 2010**

A46 Newark Bypass

Development Consent Order 202[x]

---

**Hydraulic Modelling Technical Note**

---

<b>Regulation Number:</b>	Regulation 5(2)(q)
<b>Planning Inspectorate Scheme Reference</b>	TR010065
<b>Application Document Reference</b>	7.40
<b>Author:</b>	A46 Newark Bypass Project Team, National Highways

<b>Version</b>	<b>Date</b>	<b>Status of Version</b>
Rev 1	November 2024	Deadline 3
Rev 2	February 2025	Deadline 5

## Contents

<b>About.....</b>	<b>2</b>
<b>Contents.....</b>	<b>4</b>
<b>Figures .....</b>	<b>6</b>
<b>Tables.....</b>	<b>6</b>
<b>Executive summary .....</b>	<b>9</b>
<b>1. Introduction.....</b>	<b>11</b>
1.1 Background.....	11
1.2 List of terms used.....	11
1.3 Use of the National Receptor Database in the assessment of potential flood risk impacts.....	13
1.4 Structure of the technical note.....	13
1.5 Policy context, guidance and guidelines.....	14
<b>2. Modelling uncertainty .....</b>	<b>16</b>
2.1 Introduction .....	16
2.2 Sources of modelling uncertainty .....	16
2.3 Modelling tolerance.....	17
2.4 FRA [APP-177] model proving .....	17
2.5 Additional sensitivity testing .....	17
2.6 Summary .....	18
<b>3. Design event analysis – 1% AEP plus climate change.....</b>	<b>19</b>
3.1 Introduction .....	19
3.2 Windmill Viaduct .....	19
3.3 South of Cattle Market roundabout .....	23
3.4 Embankment on floodplain between Kelham Road and Nottingham to Lincoln railway line.....	25
3.5 Design event receptor analysis .....	26
<b>4. Slough Dyke realignment .....</b>	<b>31</b>
<b>5. Climate change allowances applied in the hydraulic model.....</b>	<b>33</b>
<b>Appendix A - NRD to NPPF receptor vulnerabilities .....</b>	<b>34</b>
<b>Appendix B - Receptor analysis for low magnitude events.....</b>	<b>37</b>
B1. Introduction .....	37

B2.	50% AEP event (2-year return period event).....	37
B2.1	Summary - 50% AEP.....	38
B3.	20% AEP event (5-year return period event).....	40
B3.1	Summary – 20% AEP .....	41
B4.	5% AEP event (20-year return period event).....	42
B4.1	5% AEP receptors at Tolney Lane .....	42
B4.2	Sensitivity testing.....	45
B4.3	Summary .....	46
B5.	3.33% AEP event (30-year return period event).....	48
B5.1	Summary – 3.33% AEP .....	48
B6.	1% AEP event (100-year return period event).....	49
B6.1	1% AEP receptors at Fosse Road .....	50
B6.2	Sensitivity testing.....	50
B6.3	Summary – 1% AEP .....	52

## Figures

Figure 1: 1% AEP plus climate change. Windmill viaduct. Flood depth differences. Mitigated Scheme versus baseline. Original model.....	20
Figure 2: 1%AEP plus climate change. Windmill viaduct. Flood hazard. Original baseline.....	20
Figure 3: 1%AEP plus climate change. Windmill viaduct. Flood hazard. Mitigated Scheme.....	21
Figure 4: 1% AEP plus climate change. Windmill viaduct. Flood depth differences. Mitigated Scheme versus baseline. Original model. (this is a duplication of the depth differences shown in Figure 1 for ease of comparison with Figure 5).....	22
Figure 5: 1% AEP plus climate change. Windmill viaduct. Flood depth differences. Sensitivity test.....	22
Figure 6: 1%AEP plus climate change. Flood depth differences. Mitigated Scheme versus baseline. Original model.....	23
Figure 7: 1%AEP plus climate change. Cattle Market roundabout. Flood hazard. Original baseline.....	24
Figure 8: 1%AEP plus climate change. Cattle Market roundabout. Flood hazard. Mitigated Scheme.....	24
Figure 9: 1%AEP plus climate change. Flood depth differences. Mitigated Scheme versus baseline. Original model.....	25
Figure 10: Slough Dyke realignment – extract from AS-007 (General Arrangement Plans) Sheet 25005.....	31
Figure 11: Comparison of peak water levels through Slough Dyke with and without realignment.....	32
Figure 12: 5% AEP. Tolney Lane. Flood depth differences. Mitigated Scheme versus baseline. Original model.....	43
Figure 13: Schematic of flow mechanisms in area near Tolney Lane.....	44
Figure 14: Photo of the existing culvert to east and the associated ditch or trackway. Source: Skanska, 16/01/2025.....	45
Figure 15: 5% AEP. Tolney Lane. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.....	46
Figure 16: 1% AEP. Fosse Road. Flood depth differences. Mitigated Scheme versus baseline. Original model.....	50
Figure 17: Original 10m model domain (pink solid line) and its extension further south in the sensitivity model (purple dashed line).....	51
Figure 18: 1% AEP. Fosse Road. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.....	52

## Tables

Table 1: 1% AEP plus climate change. Flood depths differences. Mitigated Scheme versus baseline. Original model.....	30
--	----

Table 2: 1% AEP plus climate change. Flood depth differences. Mitigated Scheme versus baseline. Windmill Viaduct sensitivity test.....	30
Table 3: Peak flows for model inflows from the Upper River Trent catchment.....	33
Table 4: Lookup table for mapping of NRD class descriptions to NPPF vulnerability ..	34
Table 5: 50% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model. ....	39
Table 6: 20% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model. ....	41
Table 7: 5% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model. ....	47
Table 8: 5% AEP. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.....	47
Table 9: 3.33% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model. ....	48
Table 10: 1% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model. ....	53
Table 11: 1% AEP. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.....	53

## Executive summary

This technical note provides information regarding additional hydraulic modelling and analysis carried out to supplement the details provided in Appendix 13.2 Flood Risk Assessment of the Environmental Statement Appendices [APP-177]. The technical note also provides details to support the responses to the Relevant Representations [RR-020] from the Environment Agency, specifically discussing the issues raised in EAFR-001, EAFR-002, EAFR-007 and EAFR-009.

In assessment of fluvial flood risk impacts to and from the A46 Newark Bypass (the “Scheme”), the Flood Risk Assessment (FRA) [APP-177] considers that fluvial flood risk impacts in the 1% AEP plus climate change event from the Scheme with flood risk mitigations (the Mitigated Scheme) to sensitive receptors are negligible, in accordance with Design Manual for Roads and Bridges guidance. This technical note aims to improve the understanding of the flood risk impacts at receptors resulting from the Scheme, for the 1% AEP plus climate change event and also for the modelled lower magnitude events. Many of these impacts are very small in magnitude, with changes in flood depths in the hydraulic model results of the order of a few millimetres.

Sources of uncertainty in the hydraulic modelling results are discussed, noting that model uncertainties occur for all models and have a potential magnitude that is comparable to or greater than the very small changes in flood depth assessed in this technical note.

Within this technical note, model results are presented and discussed for the baseline and Scheme (permanent works) scenarios, and the return period events assessed for the Scheme. All presented impacts to receptors in the 1% AEP plus climate change event are negligible. This is discussed further in Section 3.5.

The Design Manual for Roads and Bridges guidance defines a change in peak flood level of +/- 10mm as having a “negligible” impact. The Environment Agency agrees with this assumption, observing<sup>1</sup> that a 10mm change in flood depth falls within model tolerance.

Sensitivity testing has been undertaken at Windmill Viaduct for the 1% AEP plus climate change event. Sensitivity testing was also undertaken at Fosse Road and Tolney Lane for lower magnitude events. **This sensitivity testing demonstrates that the Mitigated Scheme does not result in any increases in flood depths greater than 10mm at vulnerable receptors, for any modelled flood event.** Sensitivity tests discussed in this technical note are additional to, and independent of, any sensitivity testing discussed in Appendix A of the FRA [APP-177].

The conclusions of the FRA [APP-177] and the significance of effect for fluvial flood risk presented within Chapter 13 Road Drainage and the Water Environment of the Environmental Statement [APP-057] and the assessment presented within Appendix 13.1 Water Framework Directive Compliance Assessment of the Environmental Statement Appendices [APP-176] are unchanged by the additional sensitivity testing

---

<sup>1</sup> In EAFR-002 of Environment Agency Written Representations for Deadline 2 [REP2-043]

results presented within this technical note. All increases in flood levels presented for all modelled events are “negligible” as the increases in depth at vulnerable receptors are less than 10mm, in accordance with Design Manual for Roads and Bridges guidance and are therefore considered acceptable by the Applicant.



# 1. Introduction

## 1.1 Background

1.1.1 The Scheme comprises the development of a stretch of the A46 between Farndon Junction and Winthorpe Junction. The Scheme aims to upgrade an existing single carriageway road in Newark-on-Trent to a dual carriageway.

1.1.2 The Scheme requires the construction of a new carriageway that will be located alongside the existing carriageway. These associated works will require new junctions and features such as utilities, drainages, public rights of way and accesses, which will include environmental mitigation work.

1.1.3 As a part of the application for development consent, the Applicant has assessed the potential changes to flood risk due to the Scheme, to enable mitigation measures to be prepared as part of the Scheme design that comply with National Planning Policy Framework requirements. This assessment is presented in the FRA [APP-177] .

1.1.4 This technical note provides details to support the responses to the Relevant Representations [RR-020] from the Environment Agency, specifically discussing the issues raised in EAFR-001, EAFR-002, EAFR-007 and EAFR-009. Section 1.4 outlines how the sections of this technical note relate to the specific Relevant Representations.

1.1.5 In assessment of fluvial flood risk impacts to and from the Scheme, the FRA [APP-177] considers that fluvial flood risk impacts in the 1% AEP plus climate change event (the design event) from the Mitigated Scheme to sensitive receptors are “negligible”, in accordance with Design Manual for Roads and Bridges (DMRB) LA 113<sup>2</sup> and LA 104<sup>3</sup> guidance. Additional hydraulic modelling and analysis has been undertaken to help to respond to the Relevant Representations and therefore to supplement the details provided in the FRA [APP-177]. This additional modelling and analysis consisted of targeted sensitivity tests, using the hydraulic model to improve the understanding of flood risk impacts at specific receptors for specific events.

1.1.6 An additional technical note has been issued on the Floodplain Compensation Areas [REP3-035] in addition to this note, which provides further detail on the design and assessment of the floodplain compensation areas that form part of the Scheme.

## 1.2 List of terms used

1.2.1 The following terms are used throughout this technical note and are defined here for ease of reference:

---

<sup>2</sup> National Highways (2019) DMRB LA 113 – Road drainage and the water environment, Revision 1 [online] available at: LA 113 - Road drainage and the water environment (standardsforhighways.co.uk). [LA 113 - Road drainage and the water environment \(standardsforhighways.co.uk\)](https://standardsforhighways.co.uk/la-113-road-drainage-and-the-water-environment);

<sup>3</sup> National Highways (2020) DMRB LA 014 – Environmental assessment and monitoring, Revision 1 [online] available at: LA 104 - Environmental assessment and monitoring (standardsforhighways.co.uk). [LA 104 - Environmental assessment and monitoring \(standardsforhighways.co.uk\)](https://standardsforhighways.co.uk/la-104-environmental-assessment-and-monitoring)

- **Original model** – this was the hydraulic model developed and used to support the FRA [APP-177].
- **Sensitivity tests** – these are tests that have been undertaken using the original model as a base. The tests involve the enforcement of features in the model at specific locations to provide greater understanding of the modelled flood risk during specific flood events.
- **Baseline scenario** – this is the scenario representing the river and floodplain under the existing ‘baseline’ conditions prior to the development of the Scheme. Sensitivity tests have been undertaken on both baseline and Scheme scenarios in the hydraulic model in order to produce comparable results. Therefore, the baseline scenario reflects the model enforcements made in the sensitivity tests and consequently differs from the original model.
- **Mitigated Scheme** – this term was used in the FRA [APP-177] to refer to the representation of the Scheme in the hydraulic model that includes the mitigation measures that are part of the DCO application. In sensitivity tests, comparisons are made between the Mitigated Scheme and the baseline, both of which have the same local model enforcements, unless noted otherwise.
- **Enforced/enforcements** – these terms have been used throughout this technical note to refer to the model amendments, including ground features and structures, made as part of the sensitivity testing. The sensitivity tests are discussed in further detail in Section 3 for the 1% AEP plus climate change event, and in Appendix B for the 1% and 5% AEP events.
- **Receptor vulnerability** – the FRA [APP-177] considers receptor sensitivity according to the DMRB guidelines. These broadly align with flood risk vulnerability classifications provided in Annex 3 of the National Planning Policy Framework (NPPF)<sup>4</sup>. For this technical note, receptor vulnerability is expressed according to the NPPF classifications which are: Essential Infrastructure, Highly Vulnerable, More Vulnerable, Less Vulnerable and Water Compatible.
- **Flood hazard** – this term is used throughout the technical note to describe the potential risk to receptors. The hydraulic model outputs flood hazard classifications which align with those described in the Environment Agency “Flood Risks to People”<sup>5</sup> documentation. Modelled peak velocities and depths are used to classify areas of flooding as Low, Moderate, Significant or Extreme degree of hazard.

---

<sup>4</sup> National Planning Policy Framework - Annex 3: Flood risk vulnerability classification - Guidance - GOV.UK ([www.gov.uk](http://www.gov.uk))

<sup>5</sup> Environment Agency (2006) Flood Risks to People, available at: [https://assets.publishing.service.gov.uk/media/602bbc768fa8f50383c41f80/Flood\\_risks\\_to\\_people\\_-\\_Phase\\_2\\_The\\_flood\\_risks\\_to\\_people\\_methodology\\_technical\\_report.pdf](https://assets.publishing.service.gov.uk/media/602bbc768fa8f50383c41f80/Flood_risks_to_people_-_Phase_2_The_flood_risks_to_people_methodology_technical_report.pdf)

## 1.3 Use of the National Receptor Database in the assessment of potential flood risk impacts

1.3.1 The Environment Agency National Receptor Database (NRD)<sup>6</sup> classifies receptors according to Multi-Coloured Manual (MCM) codes<sup>7</sup>, class codes and class descriptions. The NRD contains point location information on properties such as their address and the type of property use. The NRD does not provide information about non-property-based receptors such as transport networks and environmental designations.

1.3.2 Due to licensing restrictions, the NRD is not widely accessible to the public as it relies on Ordnance Survey data, which is subject to restrictions regarding its release as open data. However, summary information derived from the NRD is included in various Environment Agency publications and reports. Therefore, both this technical note and the FRA [APP-177] only provide summary information from the analysis of NRD data.

1.3.3 For the purposes of this technical note, NRD class descriptions have been used to assign NPPF vulnerabilities, where possible, to all receptors within the study area. The tabulated NPPF vulnerability classification for each NRD class description is provided in Appendix A of this technical note.

1.3.4 However, not all NRD receptors have class descriptions. Specifically, any receptors with an MCM code of '999' lack class description information. Consequently, for all receptors with an MCM code of '999', aerial imagery was assessed to assign a probable land-use type and therefore vulnerability. The qualifier 'Low confidence' was added to the NPPF vulnerability for these receptors, e.g. "More Vulnerable (Low Confidence)" and "Less Vulnerable (Low Confidence)".

1.3.5 Although the NRD is very useful in assessing flood risk to receptors across the modelled area, there are potential sources of error in the location and classification of individual receptors. The NRD is a snapshot at a given moment in time, informed by underlying Ordnance Survey receptor address data and topology. Any inaccuracies noted in the review of NRD receptors are flagged in the relevant sections of this technical note.

## 1.4 Structure of the technical note

1.4.1 This technical note provides details and a narrative on the flood risk impacts arising from the Scheme. The document has been split into the following sections:

- **Section 2 – Modelling uncertainty:** A discussion of the uncertainties in the hydraulic modelling results that are important context for the consideration of flood risk impacts predicted by the model.

This section contains discussion points that are relevant for the responses for the Environment Agency Relevant Representations being considered in this technical note.

---

<sup>6</sup> Environment Agency (2014) NRD2014 Guidance Version 1, September 2015

<sup>7</sup> FHRC (2024) The Handbook MCM online (accessed Dec 2024)

- **Section 3 – Receptor impacts for the design event:** This section summarises the flood depth differences at receptors for the Mitigated Scheme versus baseline scenarios using the NRD and the NPPF receptor classifications for the 1% AEP plus climate change event.

This section provides information supporting the responses for EAFR-001 and EAFR-002 [RR-020].

- **Section 4 – Slough Dyke realignment:** This section documents a sensitivity test in relation to the Slough Dyke realignment.

This is relevant to the response for EAFR-007 [RR-020].

- **Section 5 – Climate change allowances applied in the hydraulic model:** This section discusses the assessment of a credible maximum river flow climate change scenario that was included in the FRA [APP-177].

This section provides information to support the response for EAFR-009 [RR-020].

- **Appendix A – NRD to NPPF receptor vulnerabilities:** This section presents the methodology for assigning NPPF flood risk vulnerabilities to all receptors within the study area to inform the detailed receptor analysis.
- **Appendix B – Receptor analysis for low magnitude events:** This section summarises the flood depth differences at receptors for the Mitigated Scheme versus baseline scenarios using the NRD and NPPF receptor classifications for events of lower magnitude than the 1% AEP plus climate change event. These are the 50%, 20%, 5%, 3.33% and 1% AEP events.

This section provides information to support the response for EAFR-001 and EAFR-002 [RR-020].

## 1.5 Policy context, guidance and guidelines

1.5.1 Guidance, standards, and best practice have been followed in the FRA (APP-177) and within this document, with particular reference to:

- DMRB LA 113 - Road drainage and the water environment<sup>2</sup>
- DMRB LA 104 - Environmental assessment and monitoring<sup>3</sup>
- National Planning Policy Framework (NPPF)<sup>8</sup>
- Planning Practice Guidance: Flood risk and coastal change<sup>9</sup>

---

<sup>8</sup> Ministry of Housing, Communities and Local Government (2012): National Planning Policy Framework. Available at [National Planning Policy Framework - Guidance - GOV.UK \(www.gov.uk\)](https://www.gov.uk/government/uploads/system/uploads/attachment_data/file/262412/nppf-2012.pdf)

<sup>9</sup> Department for Levelling Up (2022) Planning Policy Guidance: Flood risk and coastal change [online] Available at: [Flood risk and coastal change - GOV.UK \(www.gov.uk\)](https://www.gov.uk/government/uploads/system/uploads/attachment_data/file/101444/PPG-Flood-risk-and-coastal-change-2022.pdf)

- Nationally Significant Infrastructure Projects: Advice on the Water Framework Directive<sup>10</sup>

1.5.2 The FRA [APP-177] has been developed in accordance with DMRB LA 113 and LA 104 guidelines. DMRB provides guidance tables on receptor sensitivity, magnitude of impact and significance of effect<sup>11</sup>.

1.5.3 Guidance on receptor sensitivity which is provided within Table 3.70 of the DMRB LA 113, broadly aligns with NPPF receptor vulnerability classifications<sup>4</sup>, and examples of both are provided in Table 4.2 of the FRA [APP-177].

1.5.4 Of particular relevance to the assessment of impacts to receptors within the FRA [APP-177], Table 3.71 of DMRB LA 113 specifies a change in peak flood level of +/- 10mm to be a "negligible" impact. The Environment Agency agrees with this assumption [REP2-043] observing that a 10mm change in flood depth falls within model tolerance.

1.5.5 In accordance with DMRB guidance therefore, Tables 13-9 and 13-10 of Environmental Statement Chapter 13 Road Drainage and the Water Environment [APP-057] demonstrates that the Scheme would not result in significant adverse effects in terms of fluvial flood risk during both construction and operation. These tables have been reviewed in light of the modelling updates within this technical note and it is considered that the conclusions of Chapter 13 Road Drainage and the Water Environment of the Environmental Statement [APP-057] and Appendix 13.1 Water Framework Directive Compliance Assessment of the Environmental Statement Appendices [APP-176] are unchanged.

---

<sup>10</sup> Planning Inspectorate (2024), Nationally Significant Infrastructure Projects: Advice on the Water Framework Directive, available online at [Nationally Significant Infrastructure Projects: Advice on the Water Framework Directive - GOV.UK](#), accessed January 2025.

<sup>11</sup> Receptor sensitivity and magnitude of impact tables are provided in DMRB LA 113 Table 3.70 and Table 3.71, respectively. The significance of effect table is provided in DMRB LA 104 Table 3.8.1.

## 2. Modelling uncertainty

### 2.1 Introduction

2.1.1 This section provides an overview of the sources of modelling uncertainty that need to be understood in the context of the hydraulic modelling undertaken to support the FRA [APP-177].

2.1.2 The discussion on model uncertainty is important for background context as the flood depth differences discussed in this technical note are very small in magnitude and are often smaller than modelling uncertainties for this type of complex 1D-2D linked model.

### 2.2 Sources of modelling uncertainty

2.2.1 Two sources of modelling uncertainties that are relevant to this technical note are the modelling setup and in the behavior of the numerical solver.

2.2.2 Uncertainties in modelling setup include model inputs and model configuration. Model inputs, such as survey data, LiDAR, inflows and design geometry are data that are incorporated into the modelling relatively unchanged from various sources and all come with some level of error. However, the errors in these data are typically accepted on the basis that the best available data has been used for the model development.

2.2.3 During the development of a hydraulic model, there are occasions when decisions must be made regarding the best way to incorporate the input data into the model configuration. This may require interpretation of survey data, combining conflicting geometry sources, adapting design details for representation inside the model's numerical mesh and enforcing ground features and structure elevations.

2.2.4 In the development of the original hydraulic model, a conservative approach was used for decisions on the model representation in order that it captured the worst-case impact. This technical note revisits locations where a conservative approach was initially taken and refines them with a modified approach for proposed structures and other surface topographical features that might affect flow paths or flood risk.

2.2.5 The large scale and complexity of the linked watercourse and floodplain components of this model are relevant in relation to this point. The large area represented meant the decisions had to be made on the level of resolution in the model. While the model provides an appropriate tool for evaluating the Scheme, uncertainties arise from inevitable modelling choices such as the selection of the grid cell size(s). This, in turn, limits the level of detail in assessing localised flood risk impacts as the grid size governs the approaches for the representation of ground features and structures in the model.

2.2.6 Modelling uncertainties due to numerical solver behaviour can arise due to poor convergence, threshold condition impacts and localised flow conditions that do not fit comfortably inside the limitations of the computational model solver's numerical schemes (for TUFLOW, this would include conditions such as fast, deep flows which stretch the assumption that a 2D shallow water equation is applicable).



2.2.7 Numerical solver uncertainties generally have a lesser impact on the flood risk outputs of hydraulic models than model input and configuration uncertainties. However, they are relevant to this technical note because they can cause localised fluctuations and water level differences in areas away from the parts of the model that have been updated with the Scheme geometry and where the flood risk would otherwise be independent of the Mitigated Scheme.

## **2.3 Modelling tolerance**

2.3.1 The consequence of numerical solver uncertainty, which may stem from solver approach or convergence difficulties, is that the Flood Modeller and TUFLOW hydraulic modelling software will undertake multiple iterations to converge to within a specified tolerance in water level, or to minimise mass balance error.

2.3.2 Flood Modeller has a default tolerance of 0.01m (10mm) in water level and, according to the Environment Agency report SC120002, “Benchmarking the latest generation of 2D hydraulic packages” (2013), TUFLOW exhibits differences in water level compared to other packages of between 0.01m (10mm) and 0.05m (50mm), or up to 10% of the water depth. It may therefore be expected that there is an inherent level of uncertainty in model outputs. It should be noted that the DMRB guidance adopts a pragmatic approach by defining a change in peak flood level of +/- 10mm as having a “negligible” impact. The Environment Agency agrees with this assumption [REP2-043], observing that a 10mm change in flood depth falls within model tolerance.

2.3.3 It should be noted that where convergence difficulties arise, oscillations may be induced in the water surface, even though the model is achieving a desired level of mass balance. There would therefore be areas over which the baseline and Mitigated Scheme model scenarios exhibit such oscillations differently and comparing their peak water levels will expose the effect tolerances as a striped or dappled pattern in the depth comparison figures.

## **2.4 FRA [APP-177] model proving**

2.4.1 Hydraulic modelling was used to support the flood risk assessment of the Mitigated Scheme ( FRA [APP-177]). The hydraulic modelling included sensitivity testing to understand the impact of assumptions, including changes in hydraulic roughness, adjustments to inflows, blockages applied to structures and adjustments to weir coefficients. The model was subsequently calibrated and was then signed off by the Environment Agency (email correspondence with Paul Goldsmith, 1 February 2024) with regards to its technical function.

2.4.2 As discussed in the FRA [APP-177], modelling instabilities have been observed by way of localised velocity and depth fluctuations in the modelling results in locations away from the area of interest. However, these numerical uncertainties were deemed in the FRA [APP-177] not to reflect flood risk changes due to the Scheme.

## **2.5 Additional sensitivity testing**

2.5.1 This technical note focusses on how sensitive flood risk impacts at specific enforcement points are to changes in the representation of components within the original hydraulic model.

2.5.2 Sensitivity testing was undertaken on the 1% AEP plus climate change event (the design event) to further investigate flood risk increases at specific locations. These consist of alternative representations of design details at:

- Windmill Viaduct, discussed in Section 3, undertaken to assess flood risk impacts to receptors where the original model predicts increases greater than 0.01m (10mm).
- Slough Dyke, discussed in Section 4, undertaken to determine what impact the realignment of the Slough Dyke watercourse would have on predicted flood risk.

2.5.3 Additional sensitivity testing was also undertaken for the 1% AEP and 5% AEP flood events to assess flood risk impacts to receptors where the original model predicts increases greater than 0.01m (10mm). These tests consisted of the following:

- Tolney Lane, discussed in Appendix B4, undertaken to assess flood risk impacts for the 5% AEP event.
- Fosse Road, discussed in Appendix B6, undertaken to assess flood risk impacts for the 1% AEP event.

2.5.4 The outcomes of the sensitivity tests demonstrate that the Scheme has a negligible impact on flood risk for the design event (1% AEP plus climate change event) and all modelled lower magnitude flood events.

## 2.6 Summary

2.6.1 This technical note details the flood depth increases at receptors in the Mitigated Scheme and assesses the potential reasons for these increases through sensitivity testing for the design event (1% AEP plus climate change event) and all modelled lower magnitude flood events.

2.6.2 The presentation and analysis of flood risk impacts below 0.01m (10mm) helps to provide a fuller picture of the model results when considering the impact of the Mitigated Scheme on flood risk. It should be noted that increases in flood depths less than 0.01m (10mm) are considered “negligible” impacts in accordance with DMRB guidance. The Environment Agency agrees with this assumption [REP2-043], observing that a 10mm change in flood depth falls within model tolerance.

2.6.3 The sensitivity testing consisted of enforcements to ground features and structures in the original hydraulic model at specific locations. The objective of the sensitivity testing was to assess whether predicted flood risk impacts to specific receptors in the model could be reduced or removed. It is important to note that while enforcements in the model representation have been made, these adjustments are only intended to test the model’s sensitivities and the predicted impacts on receptors.



## 3. Design event analysis – 1% AEP plus climate change

### 3.1 Introduction

3.1.1 To provide further context on the Scheme's approach to passing the Exception Test, this section of the technical note provides further analysis on the Scheme design event, in addition to that provided in the FRA [APP-177]. It looks at receptor impacts at locations 4 (Windmill Viaduct), 11 (Cattle Market roundabout) and 8 (Embankment on floodplain between Kelham Road and Nottingham to Lincoln railway line) in further detail to expand upon information provided in the FRA [APP-177].

3.1.2 DMRB guidance defines a change in peak flood level of +/- 10mm as having a "negligible" impact. The Environment Agency agrees with this assumption [REP2-043], observing that a 10mm change in flood depth falls within model tolerance.

3.1.3 Section 3.5 looks at receptor impacts in further detail for the design event in line with DMRB guidance.

3.1.4 Receptor impacts for lower magnitude flood events were also analysed and details are provided in Appendix B of this technical note.

### 3.2 Windmill Viaduct

#### 3.2.1 Original model

3.2.1.1 In the 1% AEP plus climate change event, flood depth increases between 0.005m and 0.01m (5mm-10mm) are predicted west of Windmill Viaduct<sup>12</sup> on the right bank of the River Trent (Figure 1). Despite the predicted increase in flood depths, the flood hazard classification is not predicted to change between the baseline and Mitigated Scheme, remaining "Significant" (Figure 2 and Figure 3). Furthermore, changes in peak flood level less than 10mm such as at this location are considered "negligible" impacts in accordance with DMRB guidance.

3.2.1.2 The increase in depth west of Windmill Viaduct is caused by the representation of its extension on the right bank of the River Trent in the Mitigated Scheme model scenario. The representation of the embankment footprint in the original Mitigated Scheme model was conservatively estimated. Although the change in peak flood level at this location is considered 'negligible', sensitivity testing of the embankment footprint was undertaken to determine if a more detailed representation affected the assessment, and this is discussed further in Section 3.2.2.

---

<sup>12</sup> Windmill Viaduct is marked as Location 4 in Figure 8.1 of in Chapter 6.3 Environmental Statement - Appendix 13.2 Flood Risk Assessment (APP-177)

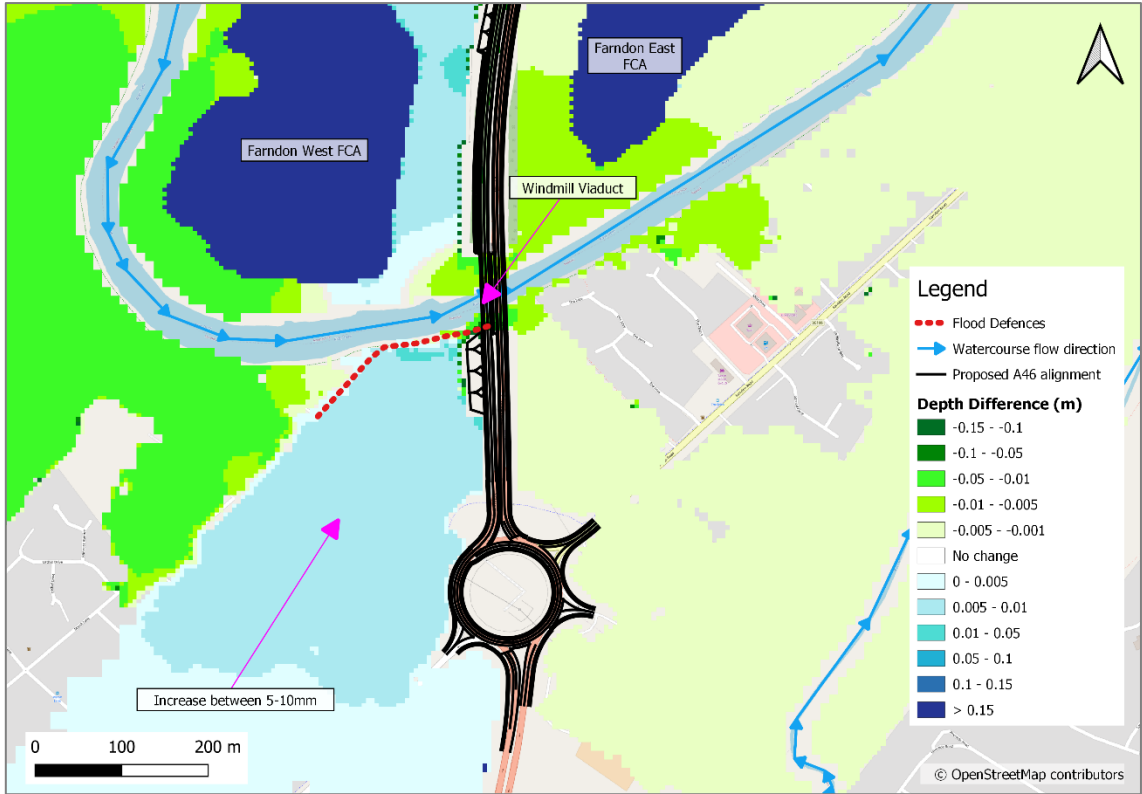


Figure 1: 1% AEP plus climate change. Windmill viaduct. Flood depth differences. Mitigated Scheme versus baseline. Original model.

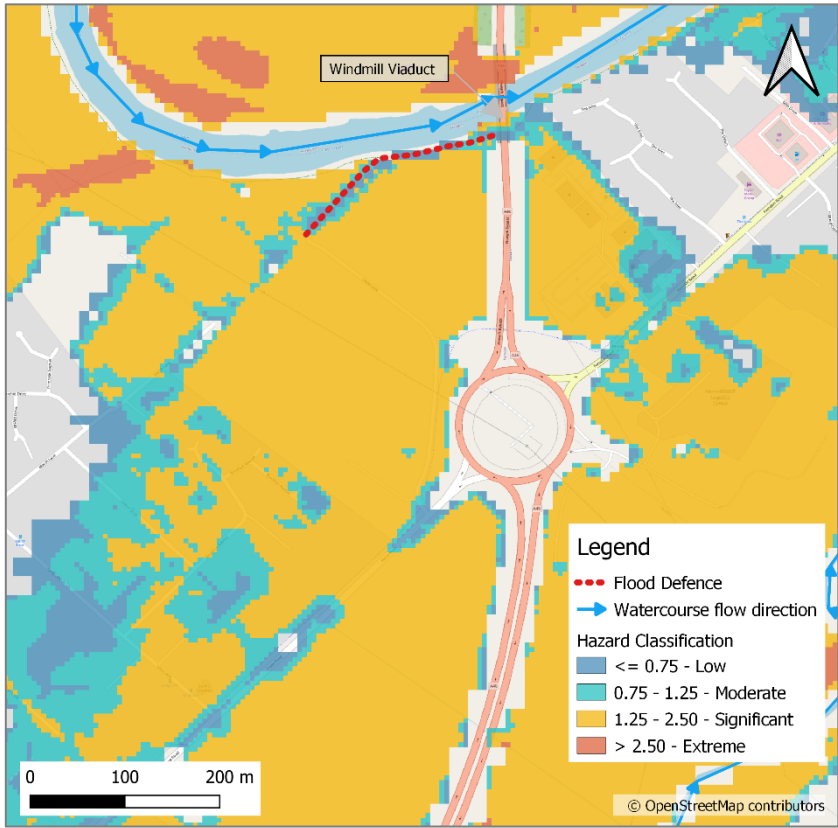


Figure 2: 1%AEP plus climate change. Windmill viaduct. Flood hazard. Original baseline.

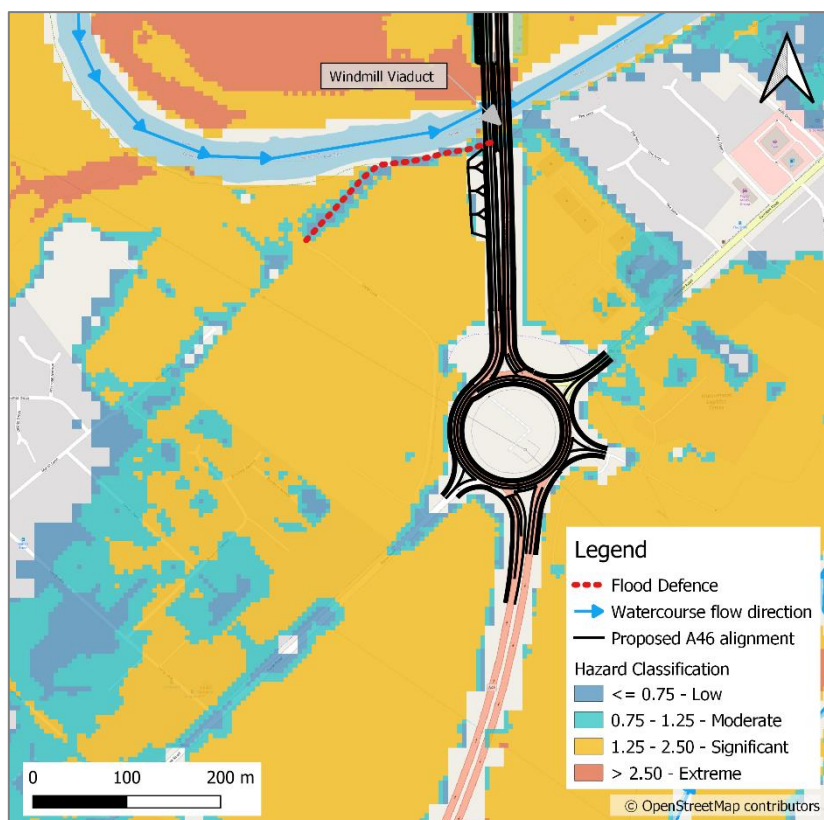


Figure 3: 1%AEP plus climate change. Windmill viaduct. Flood hazard. Mitigated Scheme.

### 3.2.2 Sensitivity testing

3.2.2.1 In the original model, a conservative approach was taken with respect to the representation of the embankment, in order to provide a conservative estimate of peak water levels in the floodplain. In the sensitivity test, the elevation of four 10m<sup>2</sup> grid cells at the northern end of the embankment was reduced, which allowed a small but significant increase in conveyance beneath the viaduct on the right bank of the River Trent, bringing the water levels and flows through the viaduct back towards baseline conditions. This test indicates the sensitivity of the model to the adjustment of just four grid cells in this area.

3.2.2.2 As a result of the modified representation of the Scheme embankment and abutment, sensitivity testing demonstrates that the area south of Windmill Viaduct now shows flood depth differences less than 0.002m (2mm) compared to the baseline (Figure 5, with Figure 4 enabling direct comparison with the depth differences from the original model as shown in Figure 1). The area of depth increase has also reduced. Detailed analysis of the results of this sensitivity test is provided in Section 3.5.

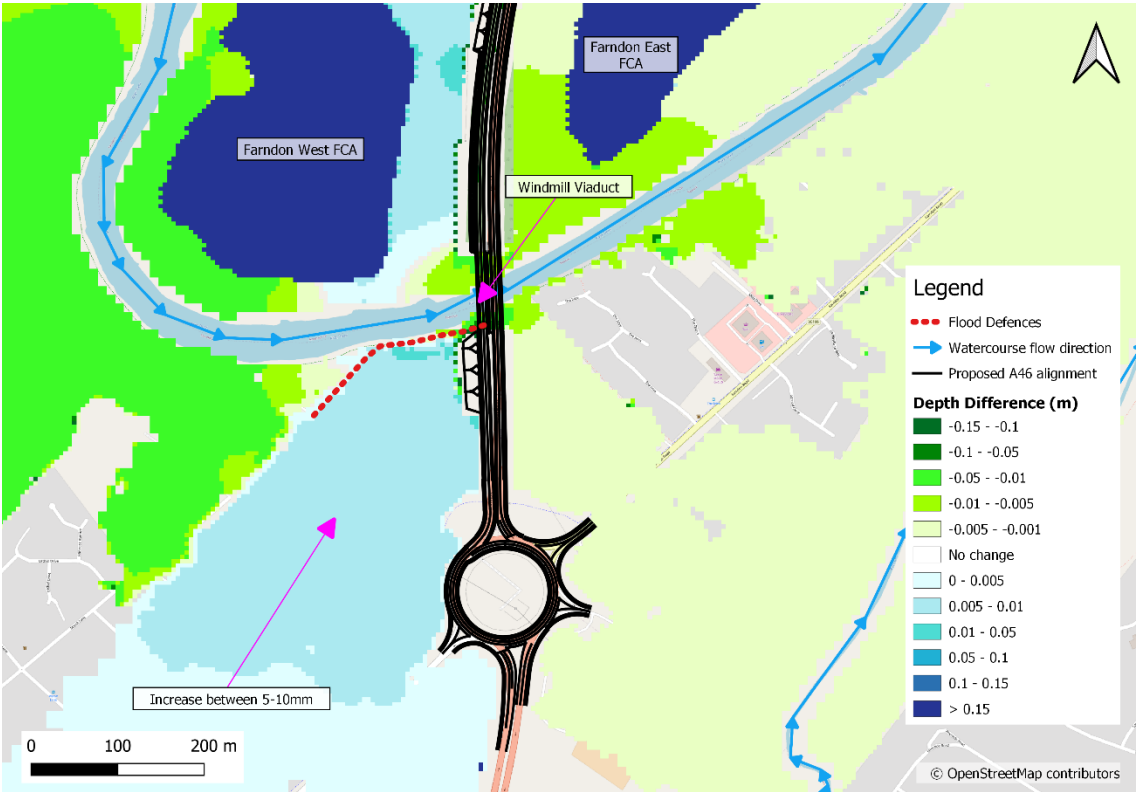


Figure 4: 1% AEP plus climate change. Windmill viaduct. Flood depth differences. Mitigated Scheme versus baseline. Original model. (this is a duplication of the depth differences shown in Figure 1 for ease of comparison with Figure 5)

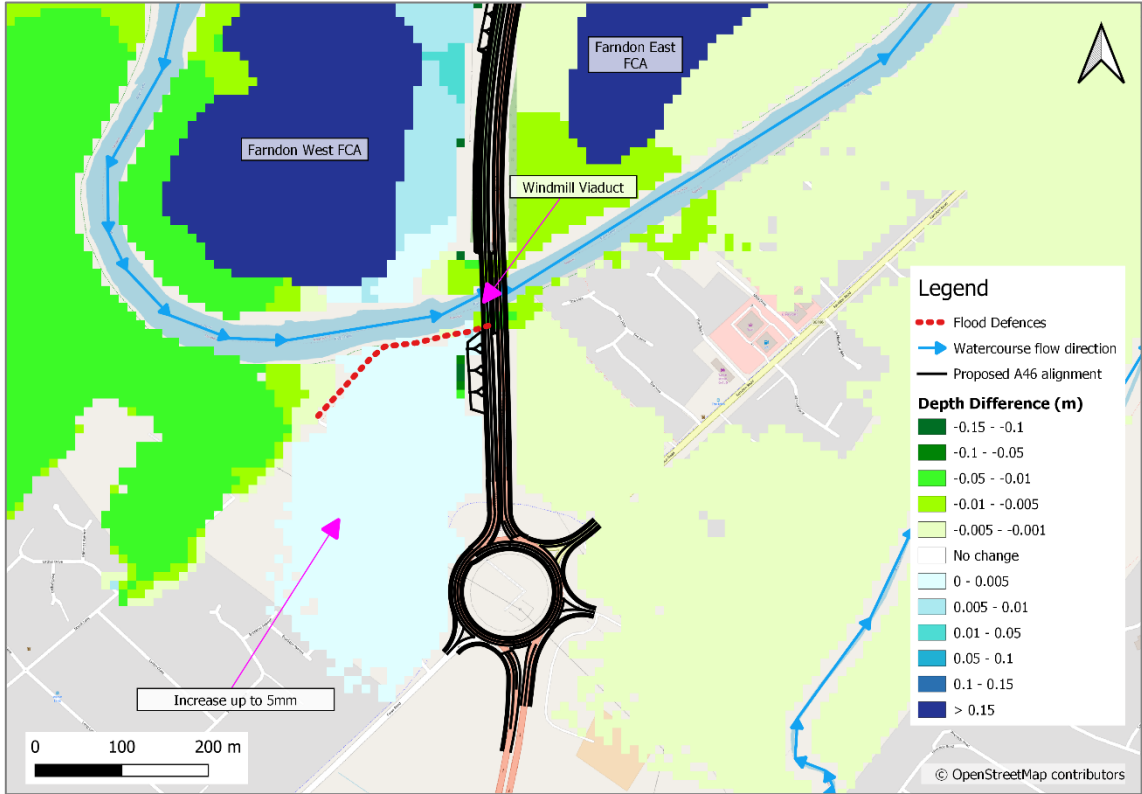


Figure 5: 1% AEP plus climate change. Windmill viaduct. Flood depth differences. Sensitivity test.

### 3.3 South of Cattle Market roundabout

3.3.1 Figure 6 presents the change in flood depths in the area south of Cattle Market roundabout. The FRA [APP-177] notes an increase in water levels up to 0.02m (20mm) in the vicinity of Cattle Market<sup>13</sup>. However, this increase affects a localised area to the north-east and does not impact any vulnerable receptors.

3.3.2 Depth increases south of Cattle Market roundabout are less than 0.01m (10mm), averaging 0.006m (6mm). This increase is considered a “negligible” impact in accordance with DMRB guidance. Baseline flood depths in this area are up to 3m for this event and the baseline flood hazard classification in the area is “Significant”. The flood hazard classification is unchanged by the Mitigated Scheme as shown in Figure 7 and Figure 8.

3.3.3 No additional sensitivity tests were undertaken for this location as the design representation of the Scheme in the original model is considered appropriate. The design representation cannot therefore reasonably be modified for sensitivity testing. Nevertheless, flood depth differences within this area resulting from the Mitigated Scheme are considered a “negligible” impact in accordance with DMRB guidance.

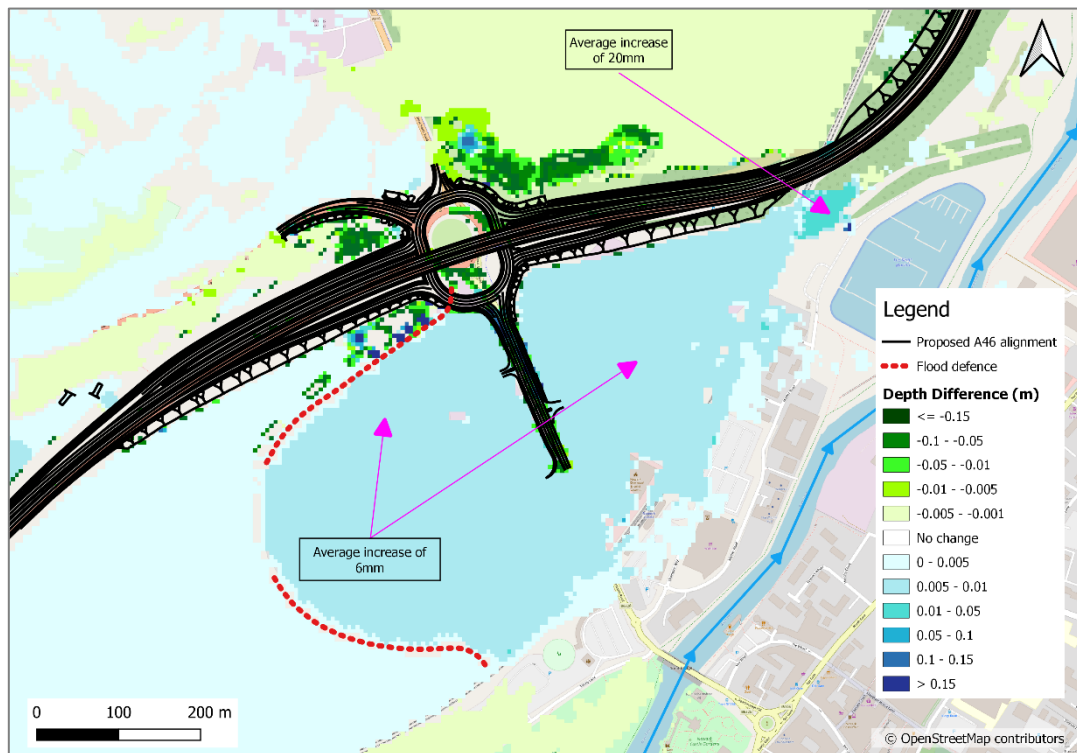


Figure 6: 1%AEP plus climate change. Flood depth differences. Mitigated Scheme versus baseline. Original model.

<sup>13</sup> The area south of Cattle Market roundabout is marked as Location 11 in Figure 8.1 of in Chapter 6.3 Environmental Statement - Appendix 13.2 Flood Risk Assessment (APP-177)



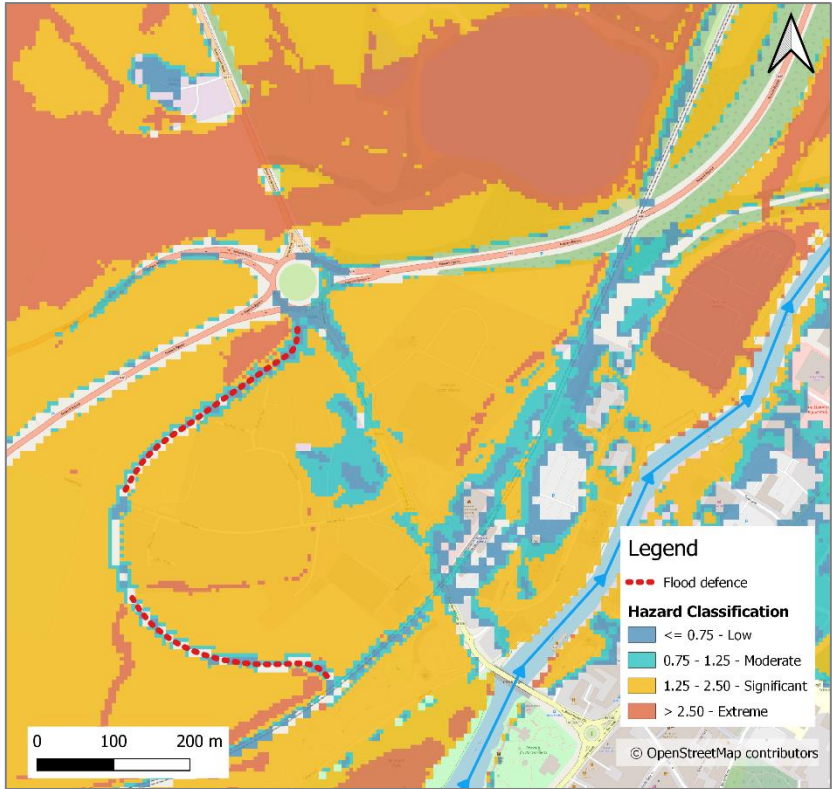


Figure 7: 1%AEP plus climate change. Cattle Market roundabout. Flood hazard. Original baseline.

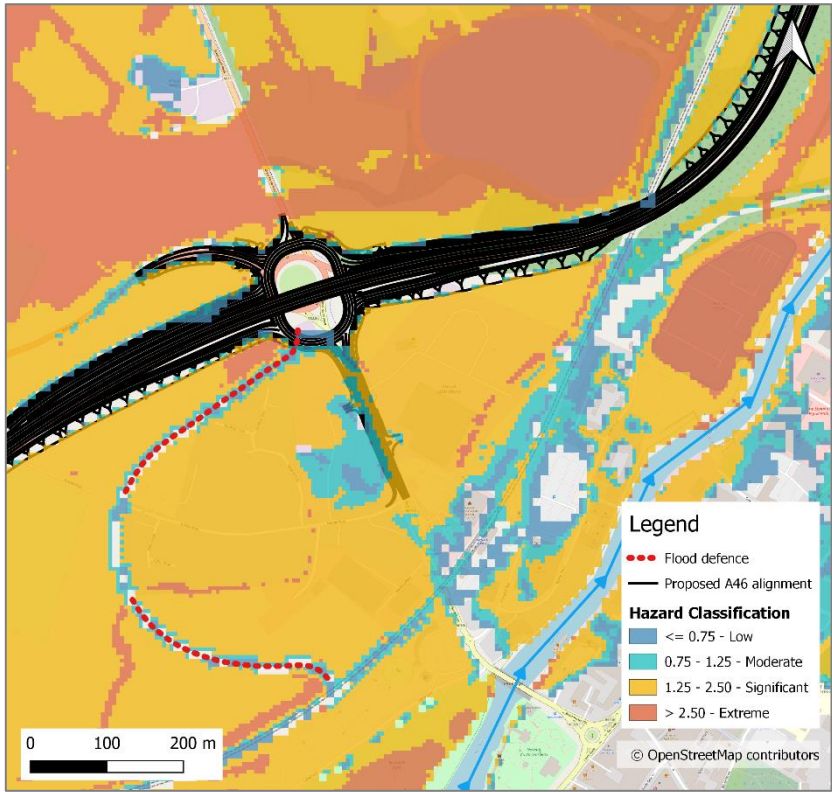


Figure 8: 1%AEP plus climate change. Cattle Market roundabout. Flood hazard. Mitigated Scheme.

### 3.4 Embankment on floodplain between Kelham Road and Nottingham to Lincoln railway line

3.4.1 The FRA [APP-177] notes that the water level at the base of the new embankment between Kelham Road and the Nottingham to Lincoln railway line<sup>14</sup> has a localised increase of up to 0.086m (86mm) from the baseline. It is important to note that this increase represents only one modelled 10m grid cell as shown in Figure 9. Elsewhere, flood depth increases are generally less than 0.01m (10mm), and decreases are also observed nearby, as shown in green in Figure 9. Changes in peak flood level less than 0.01m (10mm) are considered a "negligible" impact, in accordance with DMRB guidance.

3.4.2 There are no vulnerable receptors at this location, and the wider area is predominantly agricultural.

3.4.3 No additional sensitivity tests were undertaken for this location due to the absence of vulnerable receptors.

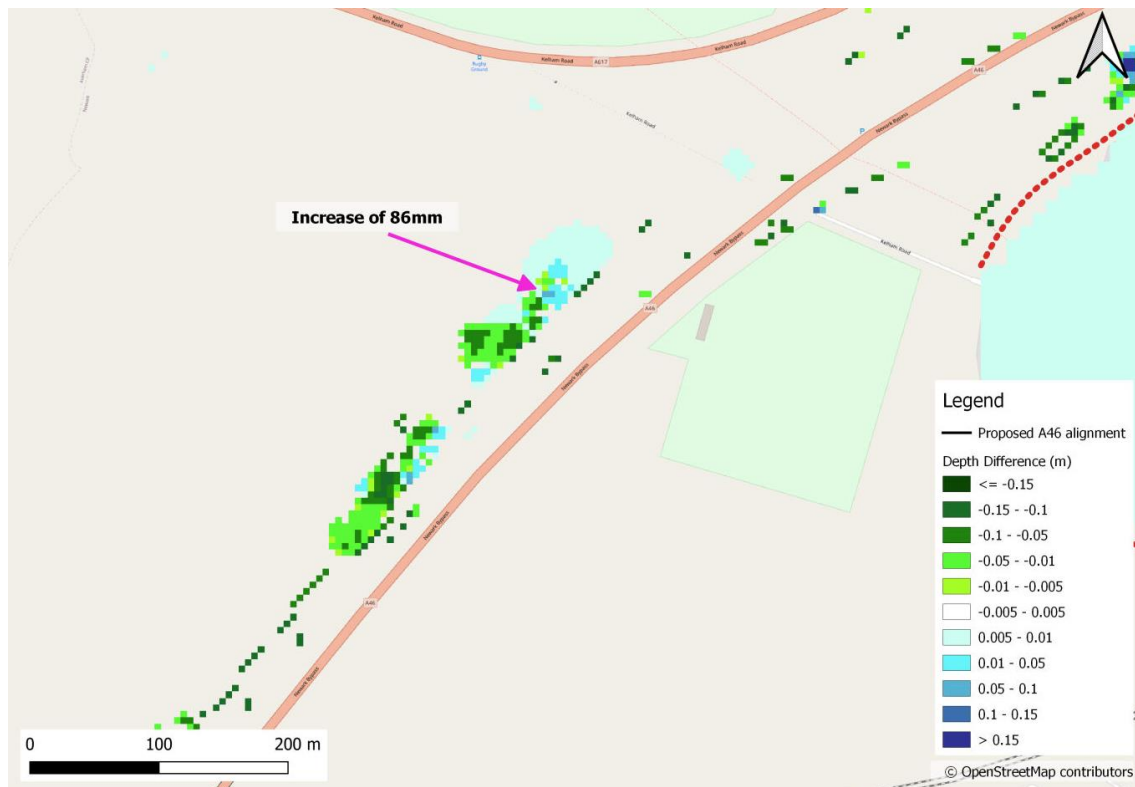


Figure 9: 1%AEP plus climate change. Flood depth differences. Mitigated Scheme versus baseline. Original model.

<sup>14</sup> The embankment between Kelham Road and Nottingham to Lincoln railway line, is marked as Location 8 in Figure 8.1 of in Chapter 6.3 Environmental Statement - Appendix 13.2 Flood Risk Assessment (APP-177)

## 3.5 Design event receptor analysis

### 3.5.1 Introduction

3.5.1.1 This section provides further detail on the changes in flood depths at receptors arising from the Mitigated Scheme for the 1% AEP event plus climate change, drawing on results of the sensitivity testing undertaken at Windmill Viaduct outlined in Section 3.2.2.

3.5.1.2 Table 1 summarises the results for the original hydraulic model. Table 2 summarises the results of the sensitivity test at Windmill Viaduct. The total numbers of receptors with increases or decreases in flood depth exclude “Water Compatible” receptors, (as identified in Appendix A of this technical note,) which by their nature are resilient to minor changes in flood level.

### 3.5.2 Original model

3.5.2.1 A comparison between the Baseline and Mitigated Scheme scenarios for the original model (Table 1) indicates that flood depths at 1,619 receptors are predicted to decrease, whilst they are predicted to increase at 1,058 receptors. Receptors with a predicted increase are summarised as follows:

- Seven “Essential Infrastructure” receptors with flood depth increases less than 0.01m (10mm) as outlined in the following points. Note that the flood hazard is not predicted to change at any of these receptors due to the Mitigated Scheme.
  - One electricity sub-station at the model boundary 5km north of the Scheme which shows a flood depth difference of 0.0004m (0.4mm) on top of a baseline depth of 0.08m (80mm).
  - One electricity sub-station at South Muskham 1.5km north of the Scheme which shows depth differences of 0.0001m (0.1mm) on top of a baseline flood depth of 0.92m (920mm).
  - One electricity sub-station located south of Cattle Market roundabout just off the Great North Road which shows a depth difference of 0.007m (7mm) on top of baseline flood depths of 0.27m (270mm).
  - Two electricity sub-stations located south of Windmill viaduct near Fosse Road which show depth differences of less than 0.002m (2mm) on top of baseline flood depths exceeding 0.26m (260mm).
  - One sewage pumping station located south of Windmill viaduct near Fosse Road which shows a flood depth difference of 0.001m (1mm) on top of a baseline flood depth of 0.90m (900mm).
  - One wind turbine, however the physical location of the turbine as observed on satellite imagery is approximately 250m from the assigned NRD receptor location, and no depth differences are observed at this location in this event.
- Seven “Highly Vulnerable” receptors with flood depth increases of less than 0.01m (10mm) as outlined in the following points. Note that the flood hazard is not predicted to change at any of these receptors due to the Scheme.
  - Two telecommunications cabinets with depth differences less than 0.007m (7mm) on top of baseline flood depths of greater than 0.05m (50mm).



- Five telecommunications infrastructure, including a cabinet and a mast with depth increases of less than 0.001m (1mm), where baseline flood depths are up to 0.64m (640mm).
- 171 “More Vulnerable” receptors with flood depth increases between 0.005 to 0.01m (5mm-10mm) as below.
  - 123 receptors are located south of Cattle Market roundabout. The average depth increase at the Cattle Market roundabout receptors is 0.006m (6mm) on top of an average baseline depth of 0.965m (965mm). Note that the flood hazard is not predicted to change at any of these receptors due to the Scheme.
  - 48 receptors are located upstream of Windmill Viaduct near Fosse Road, where the average depth increase is 0.006m (6mm) on top of an average baseline depth of 0.44m (440mm). Note that the flood hazard is not predicted to change at the majority of these receptors due to the Scheme, aside from at four receptors where it increases from either “Low” to “Moderate” or “Moderate” to “Low”.
- 25 “More Vulnerable (Low Confidence)” receptors with flood depth increases between 0.005m and 0.01m (5mm-10mm) as below. Flood depth increases for these receptors are identical to those discussed for “More Vulnerable” receptors for this event;
  - 12 receptors are located south of Cattle Market roundabout.
  - 13 receptors are located upstream of Windmill Viaduct near Fosse Road.

3.5.2.2 Note that the above increases in flood depths are less than 10mm and are therefore considered “negligible” impacts in accordance with DMRB guidance. No “More Vulnerable”, “Highly Vulnerable” or “Essential Infrastructure” receptors show flood depth increases of greater than 0.01m (10mm).

3.5.2.3 There are flood depth increases greater than 0.01m (10mm) at two “Less Vulnerable” receptors. These are the Farndon West and East FCAs. This change is expected, as the FCAs are designed to fill up and store water within the floodplain.

3.5.2.4 The model predicts that there would generally be no change in the flood hazard at receptors where an increase in flood depth is predicted, with the exception of four receptors upstream of Windmill Viaduct. The sensitivity of the receptors at this location have been assessed further and the outcomes are discussed in Section 3.5.3.

### 3.5.3 Sensitivity testing

3.5.3.1 A sensitivity test was undertaken on the 1% plus climate change event which involved amendments to the abutment and embankment representation in the Mitigated Scheme model, as discussed in Section 3.2.2. The purpose of this test was to determine whether flood depth changes at the 48 receptors upstream of Windmill Viaduct were sensitive to these amendments. The baseline for this sensitivity test was that of the original model.

3.5.3.2 The outcomes of the sensitivity test, presented in Table 2, are as follows:

- As per the original model, no “More Vulnerable”, “Highly Vulnerable” or “Essential Infrastructure” receptors show flood depth increases of greater than 0.01m (10mm).
- The number of “More Vulnerable” receptors with an increase between 0.005m to 0.010m (5mm-10mm) has reduced by 27 from 171 to 144. Following the sensitivity test, there are no longer any “More Vulnerable” receptors where depth increases above 5mm are predicted upstream of Windmill Viaduct.
- The number of receptors showing depth differences greater than 2mm has reduced, with smaller depth changes now predicted following the sensitivity test.
- Seven “Essential Infrastructure” locations with flood depth increases less than 0.01m (10mm), six of which are electricity sub-stations:
  - One at the model boundary 5km north of the Scheme which shows a flood depth difference of 0.0004m (0.4mm) on top of a baseline depth of 0.08m (80mm).
  - Two in South Muskham over 1.5km from the Scheme showing maximum depth differences of between 0.0003m to 0.0004m (0.3-0.4mm) on top of baseline flood depths between 0.57m to 0.92m (570mm-920mm).
  - One in North Muskham over 2km from the Scheme which shows maximum depth differences of 0.0001m (0.1mm) on top of a baseline flood depth of up to 0.83m (830mm).
  - One south of Cattle Market roundabout just off the Great North Road which shows a depth difference of 0.007m (7mm) on top of baseline flood depths of 0.27m (270mm).
  - One located 800m from the sewage works at Quibells Lane which shows a depth difference of 0.0001m (0.1mm) on top of baseline flood depths of 1.62m (1,620mm).
  - The remaining “Essential Infrastructure” receptor is a wind turbine according to the NRD. However, the physical location of the turbine as observed on satellite imagery is approximately 250m from the assigned NRD receptor location, and no depth differences are observed at this location in this event.
- Six “Highly Vulnerable” receptors:
  - Four telecommunications cabinets with depth differences ranging from 0.0004m to 0.0081m (0.4mm-8mm), on top of baseline flood depths of 0.03m to 0.29m (30-290mm).
  - One phone mast, with a flood depth difference of 0.0007m (0.7mm) on top of baseline flood depth of 0.19m (190mm).
  - One caravan at Tolney Lane, which sees an increase of flood depths of 0.0005m (0.5mm) on top of baseline flood depths of 0.50m (500mm). The baseline flood hazard classification at this location is “Significant” and does not change as a result of the Scheme or the sensitivity test.

3.5.3.3 For the majority of receptors other than those itemised below, the hazard classification for receptors in the sensitivity test does not change compared to the baseline.

- Hazard decrease from “Moderate” to “Low” at three “More Vulnerable” receptors.
- Hazard decrease from “Significant” to “Moderate” at one “More Vulnerable” and one “Less Vulnerable (Low Confidence)” receptors.
- Hazard increase from “Low” to “Moderate” at one “Less Vulnerable” receptor.
- Hazard increase from “Significant” to “Extreme” at water features Farndon East FCA and Farndon West FCA. Since the FCAs are designed to fill up and store water within the floodplain, this is expected.

### 3.5.4 Summary

3.5.4.1 During the 1% AEP event plus climate change, reductions in flood depth are predicted at 1,619 receptors. Furthermore, the Scheme is not predicted to cause increases in flood depths above 0.01m (10mm) at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)”. Increases greater than 0.01m (10mm) are predicted at two “Less Vulnerable” receptors, however these are the Farndon West and East FCAs and are expected. Therefore, the flood risk impacts arising due to the Scheme during the 1% AEP plus climate change event are considered “negligible” in accordance with the DMRB guidance.

3.5.4.2 The original model predicts depth increases between 5mm and 10mm at 171 “More Vulnerable” receptors, 123 of these are located south of Cattle Market roundabout and 48 are located south of Windmill Viaduct. Although increases of less than 0.01m (10mm) are considered a ‘negligible’ impact in accordance with DMRB guidance, sensitivity testing of the model at Windmill Viaduct was undertaken to better understand the increases at these receptors.

3.5.4.3 Amendments to the Windmill Viaduct Scheme embankment in the sensitivity model involved a modified representation of the Scheme embankment and abutment taking into consideration the 10m model grid size. This amendment led to a reduction in the number of “More Vulnerable” properties from 171 to 144 where a depth increase is predicted.

3.5.4.4 As a result of the amendments to the Scheme embankment at Windmill Viaduct, no “More Vulnerable” receptors upstream of Windmill Viaduct show a depth increase greater than 0.005m (5mm) or an increase in flood hazard.

3.5.4.5 The remaining 144 “More Vulnerable” receptors with a predicted depth increase between 0.005m to 0.01m (5 – 10mm) are all located south of Cattle Market roundabout, and the average predicted depth increase is 0.006m (6mm). Furthermore, flood hazard is not predicted to change at these receptors as a result of the Scheme. It should be noted that these increases are considered a “negligible” impact in accordance with the DMRB guidance and are on top of an average baseline depth of 0.965m (965mm) and are therefore considered acceptable by the Applicant.

Table 1: 1% AEP plus climate change. Flood depths differences. Mitigated Scheme versus baseline. Original model

	Count of receptors with change in depth							
NPPF Class	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	11	3	2	1	0	0	1	0
Highly Vulnerable	90	5	0	0	0	0	2	0
More Vulnerable	940	265	101	73	30	51	171	0
More Vulnerable (Low Confidence)	236	109	16	13	5	3	25	0
Less Vulnerable	234	46	11	6	1	0	44	2
Less Vulnerable (Low Confidence)	108	53	1	1	0	0	17	0
Water Compatible	213	54	7	4	0	1	10	0
Total number with decrease (excluding water compatible)								1619
Total number with increase (excluding water compatible)								1058

Table 2: 1% AEP plus climate change. Flood depth differences. Mitigated Scheme versus baseline. Windmill Viaduct sensitivity test.

	Count of receptors with change in depth							
NPPF Class	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	10	5	1	0	0	0	1	0
Highly Vulnerable	91	4	0	0	0	0	2	0
More Vulnerable	804	315	16	2	5	3	144	0
More Vulnerable (Low Confidence)	216	126	10	0	0	0	12	0
Less Vulnerable	191	56	7	4	3	0	39	2
Less Vulnerable (Low Confidence)	94	55	6	1	0	0	12	0
Water Compatible	141	70	5	1	3	1	9	0
Total number with decrease (excluding water compatible)								1406
Total number with increase (excluding water compatible)								831

## 4. Slough Dyke realignment

4.1.1 The Slough Dyke watercourse is a designated Environment Agency Main River, a River WFD Waterbody (Slough Dyke Catchment (tributary of Trent) (GB104028053111)) and is a tributary of the River Trent. Realignment of the watercourse is proposed at the location where it crosses the Scheme alignment near Brownhills Junction.

4.1.2 The realignment would move the existing watercourse by approximately 7m to 8m to the east to be aligned closer to the A1 highway. A schematic of the Slough Dyke realignment is shown in Figure 10. Details of the cross-section plan can be found in TR010065/APP/2.6 “Engineering Plans and Sections Part 6 - Structures General Arrangements APP-14”, Sheet 12.

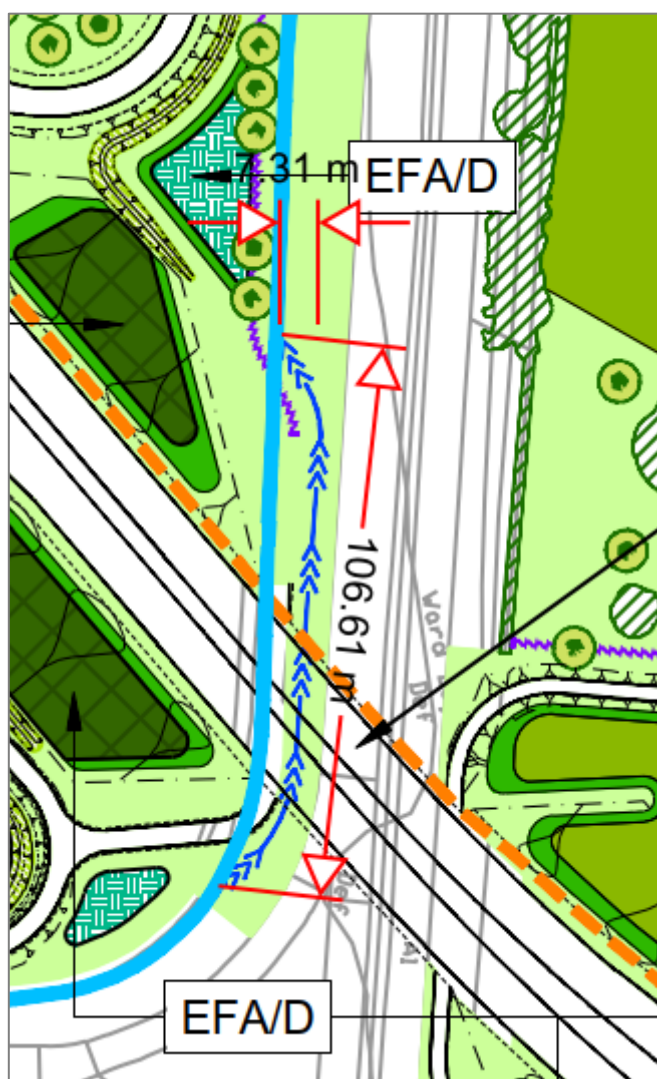


Figure 10: Slough Dyke realignment – extract from AS-007 (General Arrangement Plans) Sheet 25005

4.1.3 The existing channel cross-section shape would be retained and is not expected to change the current hydraulics or risk of flooding in the local area. The realignment was therefore not included in the original model.

4.1.4 For this technical note, sensitivity testing of the realignment has been undertaken to assess any potential change in flood risk due to the representation of the

Slough Dyke realignment. Enforcements to the hydraulic model for sensitivity testing consisted of:

- Increasing the channel length by 33m.
- Shifting the channel to the east by 8 to 10m.

4.1.5 The hydraulic model results for the 1% AEP plus climate change event demonstrate changes in peak water level of up to 0.015m (15mm) immediately upstream of the realigned section of Slough Dyke (Figure 11) within the channel. However, this has no impact on flood depths on the floodplain. It has therefore been demonstrated that the Slough Dyke realignment representation would not increase the risk of flooding. In addition, the significance of effect for fluvial flood risk presented within Chapter 13 Road Drainage and the Water Environment of the Environmental Statement [APP-057], and the assessment presented within Appendix 13.1 Water Framework Directive Compliance Assessment of the Environmental Statement Appendices [APP-176] are unchanged.

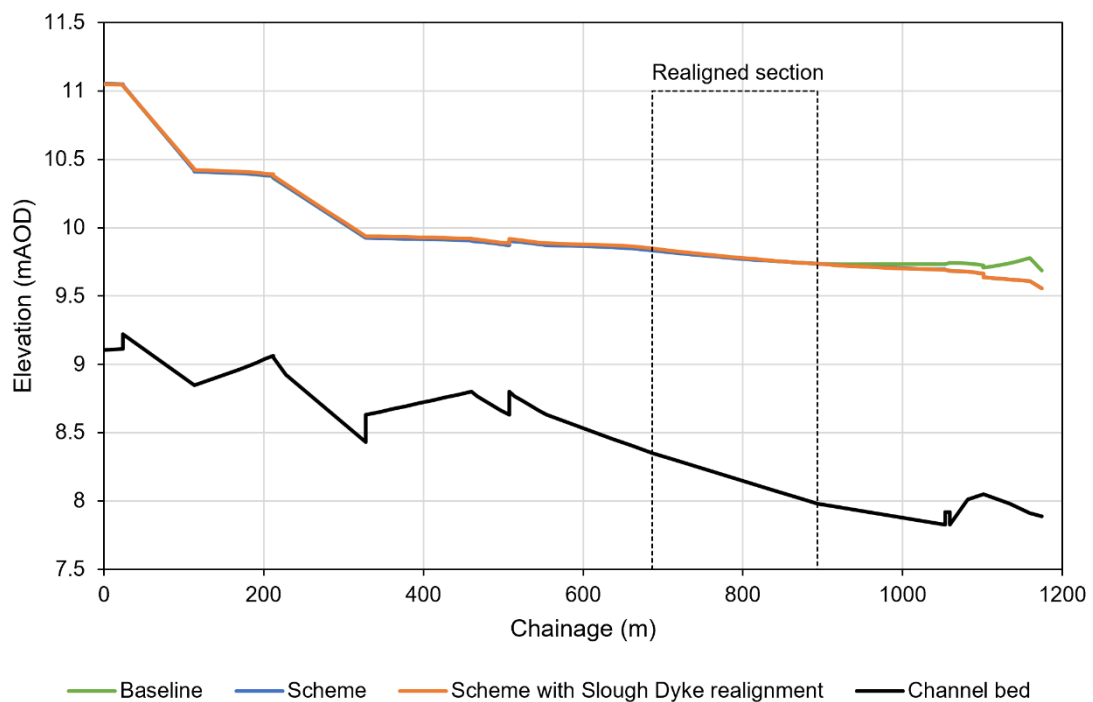


Figure 11: Comparison of peak water levels through Slough Dyke with and without realignment



## 5. Climate change allowances applied in the hydraulic model

5.1.1 The use of the hydraulic model for the assessment of the flood risk for the baseline and Mitigated Scheme scenarios for the FRA [APP-177] included consideration of a credible maximum climate change scenario (H++).

5.1.2 The design flood event assessed for the Mitigated Scheme was the 1% AEP plus climate change event, using the higher central allowance of 39% for the 2080s epoch (Section 8 of the FRA [APP-177]).

5.1.3 However, the credible maximum climate change scenario was also assessed. The event assessed was the 0.5% AEP plus the upper end climate change allowance of 62%. As discussed in Section 7.2 of the FRA [APP-177], this event was selected as the 'check event' required for assessment in the DMRB document CD356<sup>15</sup>. The 0.1% AEP event was used as a proxy event for the 0.5% plus 62% climate change uplift.

5.1.4 Table 3 shows the peak flows applied in the model for the major and dominant fluvial inflow from the Upper River Trent ('TRENT 01'). It can be seen from the table that the peak flow for the 0.5% AEP plus 62% climate change event (2028m<sup>3</sup>/s) is within 1% of the peak flow for the 0.1% AEP event (2007m<sup>3</sup>/s) and therefore is a suitable proxy event as discussed in the FRA [APP-177].

Table 3: Peak flows for model inflows from the Upper River Trent catchment

	Flood peak (m <sup>3</sup> /s) for the following AEP (%) events												
	50	20	10	5	4	3.3	2	1.3	1	1 plus 39% climate change	0.5	0.5 plus 62% climate change	0.1
TRENT 01	463	635	752	867	904	933	1018	1085	1134	1576	1252	2028	2007

5.1.5 The deck levels of the bridges and the elevations of the road surface for the main carriageway of the Scheme are preset, as the nature of the Scheme is a widening of an existing road rather than the construction of a new road. The analysis presented in the FRA [APP-177] shows that the main carriageway of the Scheme is not at flood risk for the 0.5% AEP plus 62% climate change event and therefore the Scheme is resilient to a credible maximum climate change scenario.

5.1.6 Some of the ancillary road connections to the Scheme, including Cattle Market roundabout for example, are at lower elevations than the rest of the Scheme as they tie in with existing sections of highway which are not to be altered as part of the Scheme and thus are not applicable for flood resilience.

<sup>15</sup> Design Manual for Roads and Bridges, CD 356 Design of highway structures for hydraulic action, Revision 1, Highways England, March 2020

## Appendix A - NRD to NPPF receptor vulnerabilities

NRD class descriptions have been used to assign NPPF flood risk vulnerabilities, where possible, to all receptors within the study area. NPPF receptor vulnerability is described in Annex 3: Flood risk vulnerability classification<sup>4</sup>. The table that was used to convert NRD class descriptions to NPPF vulnerabilities is provided in Table 4, and includes the following assumptions:

- Any points in the NRD where “Housetype” is “office” and “Floorlevel” is “dB” (definite basement) are assigned as “Less Vulnerable”.
- Any points in the NRD where “Housetype” is “flat” and “Floorlevel” is “dB” are assigned as “Highly Vulnerable”.
- The flood risk vulnerability classification mandates that essential utility infrastructure be categorised as “Essential Infrastructure”. Although there are 77 electricity sub-stations in the Study area, most of these are smaller sub-stations serving residential areas.
- Any points in the NRD labelled as “Caravan” are considered “Highly Vulnerable”, regardless of whether the caravan is permanent or temporary. Please note that the NRD may not have identified all caravans.

Table 4: Lookup table for mapping of NRD class descriptions to NPPF vulnerability

<b>NRD Class Description</b>	<b>NPPF vulnerability</b>
Electricity Sub-Station Power Station / Energy Production Water / Waste Water / Sewage Treatment Works	Essential Infrastructure
Ambulance Station Army Caravan Emergency / Rescue Service Fire Station Telecommunication	Highly Vulnerable
Boarding / Guest House / Bed And Breakfast / Youth Hostel Care / Nursing Home Children's Nursery / Crèche College Detached Dwelling General Practice Surgery / Clinic Health Care Services Health Centre Holiday / Campsite Holiday Let/Accommodation/Short-Term Let Other Than CH01 Hotel/Motel Landfill Medical Preparatory / First / Primary / Infant / Junior / Middle School Primary School Public House / Bar / Nightclub Residential Residential Institution Secondary / High School Self-Contained Flat (Includes Maisonette / Apartment) Semi-Detached Sheltered Accommodation Terraced Waste Management	More Vulnerable
Activity / Leisure / Sports Centre Agricultural	Less Vulnerable



<p> Agricultural - Applicable to land in farm ownership and not run as a separate business enterprise  Allotment  Amenity - Open areas not attracting visitors  Amusements  Ancillary Building  Animal Centre  Animal Services  Bank / Financial Service  Bingo Hall / Cinema / Conference / Exhibition Centre / Theatre / Concert Hall  Builders' Yard  Bus / Coach Station  Car / Coach / Commercial Vehicle / Taxi Parking / Park And Ride Site  Central Government Service  Church  Church Hall / Religious Meeting Place / Hall  Cinema  Commercial  Community Service Centre / Office  Community Services  Crane / Hoist / Winch / Material Elevator  Dentist  Equestrian  Factory/Manufacturing  Farm / Non-Residential Associated Building  Fast Food Outlet / Takeaway (Hot / Cold)  Football Facility  Forestry  Garage  Grab / Skip / Other Industrial Waste Machinery / Discharging  Grazing Land  Hopper / Silo / Cistern / Tank  Horticulture  Indoor / Outdoor Leisure / Sporting Activity / Centre  Industrial Applicable to manufacturing, engineering, maintenance, storage / wholesale distribution and extraction sites  Industrial Support  Job Centre  Land  Law Court  Leisure - Applicable to recreational sites and enterprises  Library  Local Government Service  Manufacturing  Market (Indoor / Outdoor)  Mineral / Ore Working / Quarry / Mine  Museum / Gallery  Office  Office / Work Studio  Other Licensed Premise / Vendor  Park  Permanent Crop / Crop Rotation  Petrol Filling Station  Place Of Worship  Playground  Police Box / Kiosk  Police Training  Post Office  Public / Village Hall / Other Community Facility  Public Car Parking  Public Park / Garden  Racquet Sports Facility  Railway Asset  Recreational / Social Club  Recycling Site  Restaurant / Cafeteria  Retail  Retail Service Agent  Servicing Garage  Shop / Showroom  Station / Interchange / Terminal / Halt  Steel Works </p>	
---	--

<p>Theatre Vehicle Storage Vet / Animal Medical Treatment Warehouse / Store / Storage Depot Wholesale Distribution Workshop / Light Industrial Chimney / Flue Other Educational Establishment Water Sports Facility Transport Transport Track / Way Transport Related Infrastructure Underground Feature Castle / Historic Ruin Development Site Development Dual Use</p>	
<p>Advertising Hoarding Bus Shelter Cemetery / Crematorium / Graveyard. In Current Use. Channel / Conveyor / Conduit / Pipe House Boat Lake / Reservoir Maintained Amenity Land Marina Memorial / Market Cross Monument Mooring Named Pond Object of Interest Open Space Other Utility Use Parent Shell PO Box Postal Box Property Shell Public Convenience Pump House / Pumping Station / Water Tower (water compatible) Static Water Street Record Telephone Box Tourist Information Signage Traffic Information Signage Unused Land Utility Vacant / Derelict Land Verge / Central Reservation</p>	<p>Water Compatible</p>

## Appendix B - Receptor analysis for low magnitude events

### B1. Introduction

This Appendix summarises the flood depth differences at receptors for the Mitigated Scheme versus baseline scenarios for lower magnitude events than the 1% AEP plus climate change event. The analysis uses the same methodology as used for the 1% AEP plus climate change analysis in Section 3 of this technical note.

The summary tables present the total numbers of receptors with increases or decreases in flood depth. The totals exclude “Water Compatible” receptors, as identified in Appendix A, which by their nature are resilient to minor changes in flood level.

DMRB guidance sets out flood depth thresholds only for the 1% AEP plus climate change event. Therefore, for the smaller events reported in Appendix B these thresholds (and subsequent conclusions of significance of effect) are provided for context only. Changes in peak flood level less than 0.01m (10mm) are still considered a “negligible” impact, in accordance with the DMRB guidance. The Environment Agency agrees with this assumption (REP2-043), observing that a 10mm change in flood depth falls within model tolerance. However, for the purpose of reporting, all depth increases above 0.001m (1mm) have been reported in the summary tables.

### B2. 50% AEP event (2-year return period event)

A comparison between the baseline and Mitigated Scheme scenarios for the original model (Table 5) indicates that 14 receptors are predicted to decrease in flood depths, whilst 15 receptors are predicted to increase.

No flood depth increases above 0.01m (10mm) are predicted at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)” receptors.

For vulnerable receptors, predicted increases smaller than 0.01m (10mm) and therefore considered “negligible” impacts are summarised as follows :

- One “Essential Infrastructure” receptor with a flood depth increase of less than 0.002m (2mm) compared to the baseline.
  - This is a wind turbine. However, the physical location of the turbine as observed on satellite imagery appears to be approximately 250m from the assigned NRD receptor point, and no depth differences are observed at this location.
- Three “Highly Vulnerable” receptors with flood depth differences of less than 0.005m (5mm) on top of baseline flood depths between 0.045m and 0.10m (45mm-100mm).
- Three “More Vulnerable (Low Confidence)” receptors with flood depth increases of less than 0.01m (10mm) on top of baseline flood depths between 0.02m and 0.47m (20mm-470mm). Eight “Less Vulnerable” receptors with flood depth

increases of greater than 0.005m (5mm) include the cricket club, the rugby club, five greenfield locations, and one hopper located 1.2km north of the Scheme, with baseline flood depths between 0.02m and 0.79m (20mm-790mm).

- Four “Less Vulnerable” receptors where increases in flood depths greater than 0.01m (10mm) are predicted, one of these being an increase of 0.031m (31mm) at the cricket club, and the remaining three being greenfield sites. It should be noted that at the cricket club, the baseline depth is 0.40m (400mm), and there would be no change in flood hazard due to the Scheme.

The model predicts that the Scheme would not change the flood hazard classification at most receptors other than those below:

- Increase from “Low” to “Significant” at one “Less Vulnerable” receptor at the Farndon West FCA. This change is expected, as the FCA is designed to fill up and store water within the floodplain. There is no change at the Farndon East FCA as the hazard rating is already “Significant”.
- Decrease from “Significant” to “Moderate” at one “Less Vulnerable” receptor at agricultural land near Cattle Market roundabout.

## **B2.1      Summary - 50% AEP**

During the 50% AEP event, reductions in flood depths are predicted at 14 receptors. The Scheme is not predicted to cause increases in flood depths above 0.01m (10mm) at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)” receptors.

There are four “Less Vulnerable” receptors with increases above 0.01m (10mm) due to the Mitigated Scheme, one of which is the cricket club, which has a predicted increase of 0.031m (31mm) on top of a baseline depth of 0.4m (400mm). The Mitigated Scheme model predicts no change in flood hazard at this receptor. The remaining impacted “Less Vulnerable” receptors are the two FCAs and a greenfield site.

Table 5: 50% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model.

	Count of receptors with change in depth							
NPPF Class	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	0	0	1	0	0	0	0	0
Highly Vulnerable	0	0	0	1	0	2	0	0
More Vulnerable	0	0	0	0	0	0	0	0
More Vulnerable (Low Confidence)	0	0	0	0	0	1	2	0
Less Vulnerable	14	0	0	0	0	0	4	4
Less Vulnerable (Low Confidence)	0	0	0	0	0	0	0	0
Water Compatible	54	0	0	3	5	0	9	1
Total number with decrease (excluding water compatible)								14
Total number with increase (excluding water compatible)								15

### **B3. 20% AEP event (5-year return period event)**

A comparison between baseline and Mitigated Scheme scenarios for the original model (Table 6) indicates that 44 receptors are predicted to decrease in flood depths, whilst 36 receptors are predicted to increase.

No flood depth increases above 0.01m (10mm) are predicted at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)” receptors.

For vulnerable receptors, predicted increases smaller than 0.01m (10mm) and therefore considered “negligible” impacts are summarised as follows :

- Two “Essential Infrastructure” receptors with flood depth increases less than 1mm on top of baseline flood depths between 0.20m and 0.77m (200-770mm).
  - One is a wind turbine according to the NRD. However, the physical location of the turbine, as observed on satellite imagery is approximately 250m from the assigned NRD receptor location, and no depth differences are observed at this location.
  - One is an electricity sub-station 1.2km northwest of the Scheme where the baseline flood depth is already 0.77m (770mm).
- Four “Highly Vulnerable” receptors with flood depth differences of less than 0.005m (5mm) on top of baseline depths of up to 0.35m (350mm). These receptors are caravans in the western end of the Tolney Lane area adjacent to Old Trent Dyke. Flood hazard is not predicted to change at these receptors.
- One “More Vulnerable” receptor and one “More Vulnerable (Low Confidence)” receptor at Tolney Lane with flood depth differences greater than 0.005m (5mm). Flood hazard is not predicted to change at these receptors.
  - At the “More Vulnerable” receptor, the predicted flood depth increase of 0.006m (6mm) is on top of a baseline depth of 0.14m (140mm). However, upon close inspection of the results, the respective depths in the baseline and Mitigated Scheme scenarios both show minor numerical fluctuations in this area at the peak of up to 0.005m (5mm), indicating that the modelled depth increases in this area are a result of modelling uncertainty (see Section 2) rather than a material flood risk impact.
  - Based on aerial imagery, the receptor marked as “More Vulnerable (Low Confidence)” appears unlikely to be a “More Vulnerable” residential dwelling.
- Seven “Less Vulnerable” receptors with flood depth increases of greater than 0.005m (5mm) include the cricket club, the rugby club, and five greenfield locations with baseline flood depths between 0.30m and 1.20m (300mm-1200mm). Flood hazard is not predicted to change at these receptors, aside from one which is agricultural land and is predicted to increase from “Moderate” to “Significant”.
- Three “Less Vulnerable (Low Confidence)” receptors with flood depth increases of greater than 0.005m (5mm), including the rugby club, one at the British Sugar Factory 750m north of the Scheme, and one at agricultural land 150m west of

the Scheme. Flood hazard is not predicted to change at these receptors, aside from one which is located at the British Sugar Factory and is predicted to increase from “Low” to “Moderate”.

### B3.1 Summary – 20% AEP

During the 20% AEP event, reductions in flood depths are predicted at 44 receptors. The Mitigated Scheme is not predicted to increase flood depths above 0.01m (10mm) at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)” or “Less Vulnerable (Low Confidence)” receptors.

There is one “More Vulnerable” receptor at Tolney Lane where an increase of 0.006m (6mm) is predicted. It should be noted that this is on top of a baseline depth of 0.14m (140mm) with no predicted change in flood hazard and is considered a “negligible” impact.

There are three “Less Vulnerable (Low Confidence)” receptors with increases greater than 0.01m (10mm). One of these is the cricket club, with an increase of 0.012m (12mm) on top of a baseline depth of 0.7m (700mm). Note that flood hazard is not predicted to change. The remaining two receptors are the Farndon West and East FCAs, which are designed to fill.

Table 6: 20% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model.

NPPF Class	Count of receptors with change in depth							
	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	0	2	0	0	0	0	0	0
Highly Vulnerable	0	0	0	1	0	3	0	0
More Vulnerable	4	4	0	1	0	0	1	0
More Vulnerable (Low Confidence)	9	2	0	1	0	0	4	0
Less Vulnerable	30	3	0	2	0	0	4	3
Less Vulnerable (Low Confidence)	1	0	0	2	0	0	3	0
Water Compatible	78	2	0	4	1	1	3	8
Total number with decrease (excluding water compatible)								44
Total number with increase (excluding water compatible)								36

## **B4. 5% AEP event (20-year return period event)**

A comparison between the baseline and Mitigated Scheme scenarios for the original model indicates that 201 receptors are predicted to decrease in flood depths, whilst 69 receptors are expected to increase.

No flood depth increases above 0.01m (10mm) are predicted at any “Essential Infrastructure”, “Highly Vulnerable”, or “Less Vulnerable (Low Confidence)” receptors. The model predicts that the Scheme would not change the flood hazard classification at most receptors, decreasing flood hazard at four receptors and increasing it at five.

Predicted increases smaller than 0.01m (10mm) and therefore considered “negligible” impacts are summarised as follows:

- One “Essential Infrastructure” receptor with a flood depth increase of less than 0.001m (1mm). This is an electricity sub-station located 800m northwest of the Scheme, where baseline depths are 0.65m (650mm). Flood hazard is not predicted to change at this receptor.

The model predicts flood depth increases above 0.01m (10mm), however investigation of model results indicates that changes in flood level at their locations are sensitive to model inputs. This is discussed further in Sections B4.1 and B4.2 of this Appendix. The receptors with predicted increases above 0.01m (10mm) are summarised as follows:

- Three “Less Vulnerable” with an increase greater than 0.01m (10mm), two of which are located within the Farndon East and West FCAs. This change is expected, as the FCA is designed to fill up and store water within the floodplain. The remaining “Less Vulnerable” receptor is the cricket club, where a depth increase of 0.025m (25mm) is predicted on top of a baseline depth of 0.98m (980mm), though flood hazard is not predicted to change and remains “Significant”.
- Three “More Vulnerable” receptors and two “More Vulnerable (Low Confidence)” receptors with flood depths of greater than 0.01m (10mm) and less than 0.02m (20mm) on top of baseline depths between 0.045m and 0.40m (45-400mm). These receptors are located at Tolney Lane. Flood hazard is not predicted to change at these receptors. It is noted that these receptors at Tolney Lane are sensitive to local drainage and access infrastructure for the Nottingham to Lincoln railway line, and this is discussed further in Sections B4.1 and B4.2 of this Appendix.
- One “Less Vulnerable” receptor which is not predicted to flood in the baseline is now predicted to flood due to the Scheme, this is the Riverside Car Park. The hazard classification at this location due to the Scheme is “Low”. The flood depths due to the Scheme at this location range between 0.0075m to 0.05m (7.5-50mm). This is discussed further in Sections B4.1 and B4.2 of this Appendix.

### **B4.1 5% AEP receptors at Tolney Lane**

The predicted increases greater than 0.01m (10mm) at the three “More Vulnerable” and two “More Vulnerable (Low Confidence)” receptors are located to the south of an



existing opening beneath the Nottingham to Lincoln railway line near the Tolney Lane area. This location is shown in Figure 12. It should be noted that a depth increase greater than 0.01m (10mm) does not occur at this location for any other modelled flood event.

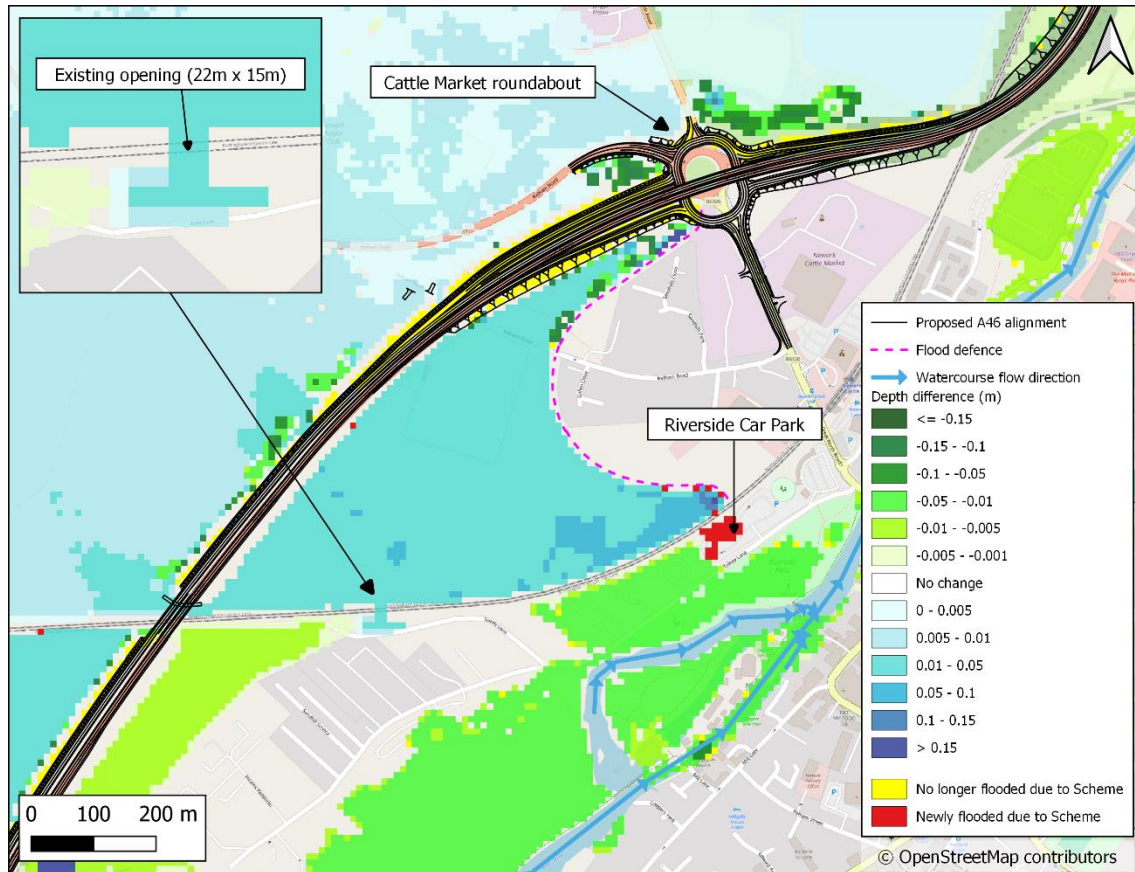


Figure 12: 5% AEP. Tolney Lane. Flood depth differences. Mitigated Scheme versus baseline. Original model.

5.1.7 The existing opening beneath the Nottingham to Lincoln railway line was inaccessible during a site visit in December 2024, and therefore the condition of the structure is unknown. Due to the size of the opening, it has been incorporated in the baseline and Mitigated Scheme models as a clear opening in the ground model which enables a conservative estimate of flow through the area, and ultimately a conservative estimate of flood risk at receptors.

5.1.8 Further investigation into the flow mechanisms at Tolney Lane and the surrounding area were undertaken for the 5% AEP event, given that this is the only event where flood depth increases above 0.01m (10mm) are predicted at this location. A schematic of the flow mechanisms in this area is shown in Figure 13 and a description is given below:

- (1) Flow enters the area behind the Kelham Road flood defence from the western floodplain via the opening beneath the A46 where it crosses the Nottingham to Lincoln railway line (1a), and an existing culvert (1b). The flow direction is west to east at this point.
- (2) Due to the Scheme embankment to the north, there is an increase in water level behind the Kelham Road flood defence in comparison to the baseline. In the area behind the Kelham Road flood defence, flow generally circulates

behind the bund. Flow is directed north towards culvert beneath Cattle Market roundabout (2), as well as to the east towards an existing culvert beneath the Nottingham to Lincoln railway line (3).

- (3) Flow moves eastwards through the area behind the Kelham Road flood defence and leaves the area via an existing culvert beneath the Nottingham to Lincoln railway line to the east where it then reaches an existing ditch or trackway that runs parallel to the Riverside way car park in a south westerly direction. The condition of this ditch or trackway is unknown, a photo of the structure is included in Figure 14 below, with the structure showing localised pluvial flooding at the time of the site visit.

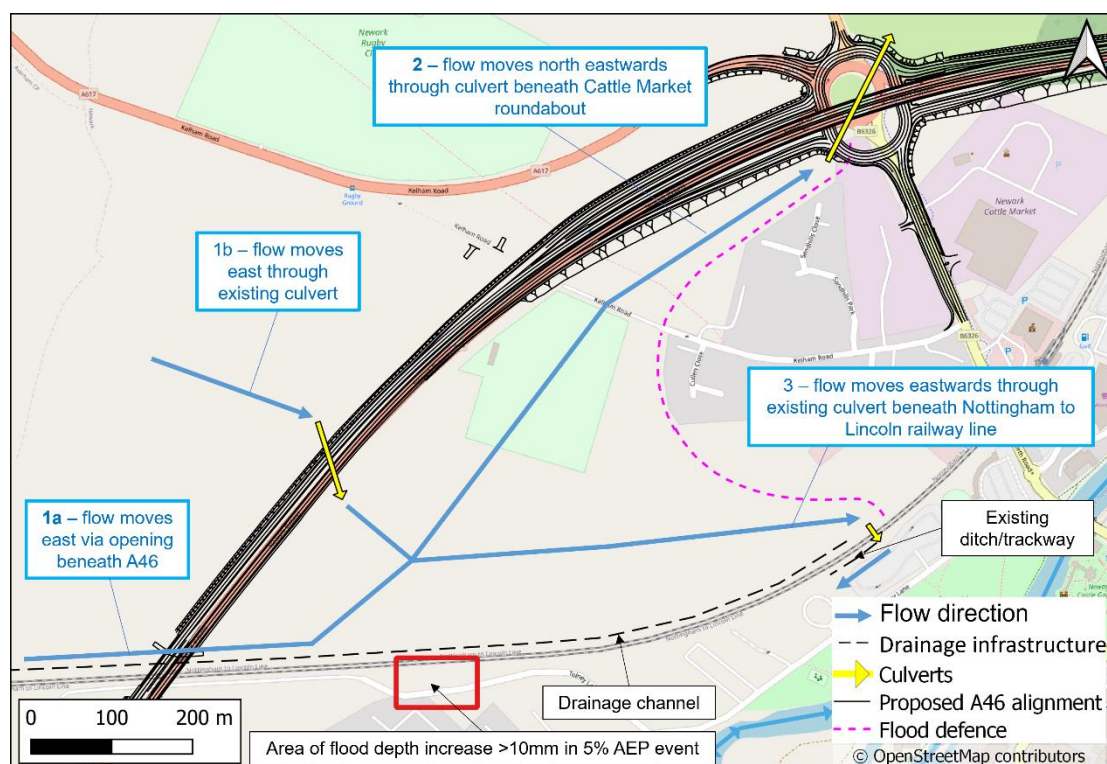


Figure 13: Schematic of flow mechanisms in area near Tolney Lane





Figure 14: Photo of the existing culvert to east and the associated ditch or trackway. Source: Skanska, 16/01/2025.

5.1.1 The representation of several elements in this region of the model has been reviewed and modified as part of sensitivity testing of this model area. This is discussed in section B4.2 of this Appendix.

## B4.2 Sensitivity testing

5.1.2 The sensitivity test consisted of the following modifications to key model elements near the Tolney Lane area:

- **Eastern culvert beneath Nottingham to Lincoln railway line:** surveyed culvert invert levels were enforced, enabling smoother conveyance of flow through the culvert and out of the area behind the Kelham Road flood defence bund.
- **Enforcement of the existing ditch or trackway to the south of the eastern culvert:** this feature was not captured in the survey, therefore LiDAR levels were used. A 50% constriction was applied to prevent overestimation of the flow area in the model. This ultimately provides a more conservative model set up for assessing flood risk at Tolney Lane.
- **Enforcement of an existing drainage channel running parallel to the north of the Nottingham to Lincoln railway line:** surveyed levels were enforced, and it should be noted that the surveyed elevations are assumed to be the water surface elevations in the drainage channel rather than the invert. This means that the representation of the drainage channel is conservative.

5.1.3 Results of the sensitivity test demonstrate that with the modifications to key elements given above, there are no depth increases due to the Scheme greater than 0.01m (10mm) near Tolney Lane. The flood depth change following the sensitivity test are shown in Figure 15.

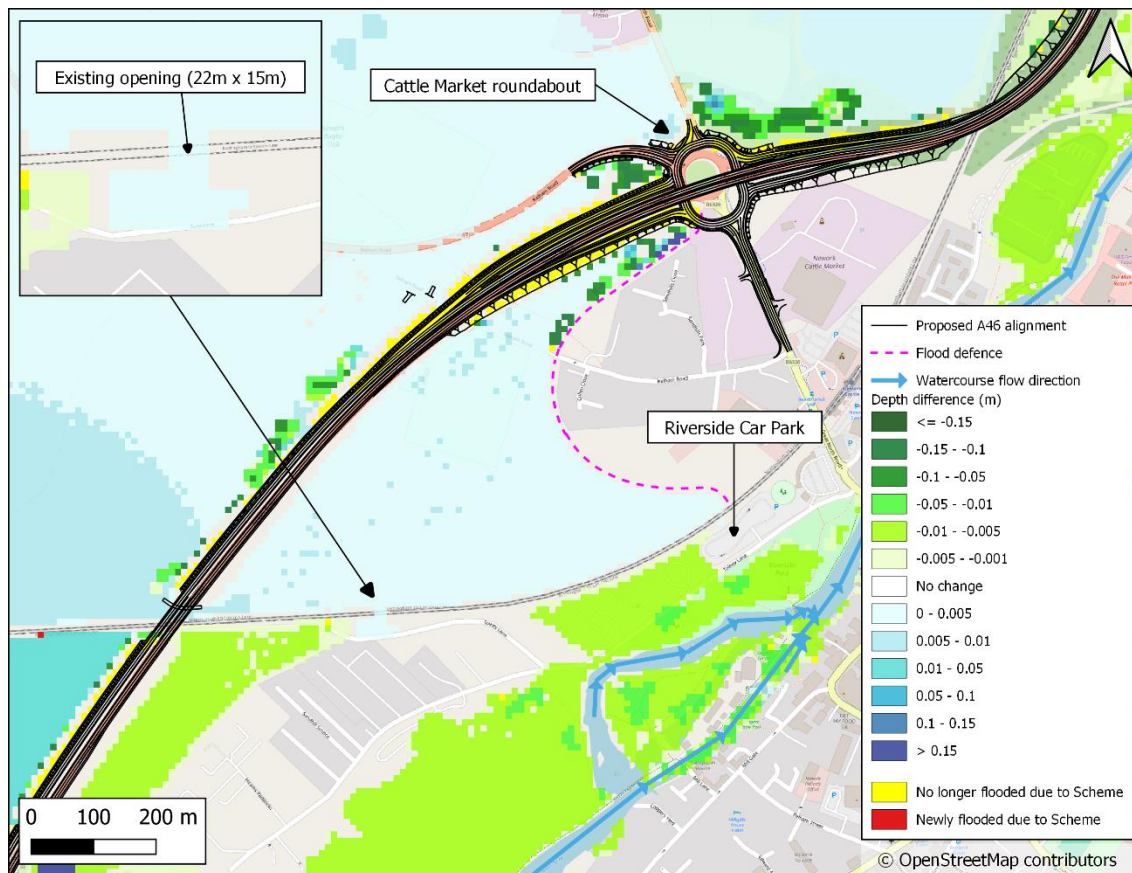


Figure 15: 5% AEP. Tolney Lane. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.

5.1.1 The sensitivity test shows no increases in flood depth greater than 0.01m (10mm) at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)” receptors. Flood depth increases at the cricket club have reduced to less than 0.01m (10mm), and the Riverside Car Park is no longer predicted to flood as a result of the Mitigated Scheme. The only predicted flood depth increases above 0.01m (10mm) are the “Less Vulnerable” FCAs which are designed to fill up and store water within the floodplain .

## B4.3 Summary

The original model predicts that flood depths would increase by more than 0.01m (10mm) at five “More Vulnerable” or “More Vulnerable (Low Confidence)” receptors at Tolney Lane. An increase greater than 10mm is also predicted at the “Less Vulnerable” cricket club. No change in flood hazard is predicted at these receptors.

Sensitivity testing was undertaken to determine if these increases were sensitive to local drainage and access infrastructure for the Nottingham-Lincoln railway line, near the Tolney Lane area. The sensitivity test at Tolney Lane shows no flood depth increases greater than 0.01m (10mm), other than at two “Less Vulnerable” receptors, which are the Farndon West and East FCAs, as expected.

Table 7 presents the receptor analysis for the 5% AEP event for the original model, and Table 8 presents the receptor analysis incorporating the sensitivity test.

*Table 7: 5% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model.*

	Count of receptors with change in depth							
<b>NPPF Class</b>	<b>&lt;0mm</b>	<b>0-1mm</b>	<b>1-2mm</b>	<b>2-3mm</b>	<b>3-4mm</b>	<b>4-5mm</b>	<b>5-10mm</b>	<b>&gt;10mm</b>
Essential Infrastructure	3	1	0	0	0	0	0	0
Highly Vulnerable	37	0	0	0	0	0	0	0
More Vulnerable	56	2	0	3	2	1	0	3
More Vulnerable (Low Confidence)	35	12	2	1	2	2	2	2
Less Vulnerable	53	12	1	1	0	1	5	4
Less Vulnerable (Low Confidence)	17	1	0	3	1	3	2	0
Water Compatible	81	16	2	3	10	1	4	0
<b>Total number with decrease (excluding water compatible)</b>								<b>101</b>
<b>Total number with increase (excluding water compatible)</b>								<b>68</b>

*Table 8: 5% AEP. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.*

	Count of receptors with change in depth							
<b>NPPF Class</b>	<b>&lt;0mm</b>	<b>0-1mm</b>	<b>1-2mm</b>	<b>2-3mm</b>	<b>3-4mm</b>	<b>4-5mm</b>	<b>5-10mm</b>	<b>&gt;10mm</b>
Essential Infrastructure	1	1	0	0	0	0	0	0
Highly Vulnerable	48	0	0	0	0	0	0	0
More Vulnerable	38	1	3	1	3	1	0	0
More Vulnerable (Low Confidence)	25	4	2	3	3	0	0	0
Less Vulnerable	40	1	2	1	3	3	0	2
Less Vulnerable (Low Confidence)	10	0	4	0	5	0	0	0
Water Compatible	70	1	14	1	4	1	0	0
<b>Total number with decrease (excluding water compatible)</b>								<b>162</b>
<b>Total number with increase (excluding water compatible)</b>								<b>43</b>

## B5. 3.33% AEP event (30-year return period event)

A comparison between the baseline and Mitigated Scheme scenarios for the original model (Table 9) indicates that 265 receptors are predicted to decrease in flood depth, whilst 73 are predicted to increase.

No flood depth increases above 0.01m (10mm) are predicted at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)” receptors.

Predicted increases smaller than 0.01m (10mm) and therefore considered “negligible” impacts are summarised as follows for vulnerable receptors:

- Three “Essential Infrastructure” receptors with increases less than 0.001m (1mm), all of which are electricity sub-stations. Two of the sub-stations are at the model boundary 2.5km southeast of the Scheme, the third is located 1.5km north of the Scheme. Baseline flood depths across the three locations ranges from 0.37m (370mm) to 0.79m (790mm), and flood hazard is not predicted to change.
- Two “Less Vulnerable” receptors with increases above 0.01 (10mm). These are the Farndon West and East FCAs. This change is expected, as the FCA is designed to fill up and store water within the floodplain.

### B5.1 Summary – 3.33% AEP

During the 3.33% AEP event, reductions in flood depths are predicted at 265 receptors. The Scheme is not predicted to increase flood depths to any receptors above 0.01m (10mm), aside from two “Less Vulnerable” receptors which are the Farndon West and East FCAs as expected. In addition to this, there are no instances of increased hazard due to the Mitigated Scheme.

Table 9: 3.33% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model.

NPPF Class	Count of receptors with change in depth							
	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	2	3	0	0	0	0	0	0
Highly Vulnerable	49	0	0	0	0	0	0	0
More Vulnerable	71	10	3	3	2	1	0	0
More Vulnerable (Low Confidence)	50	12	5	2	3	1	0	0
Less Vulnerable	71	5	2	1	4	0	2	2
Less Vulnerable (Low Confidence)	22	1	5	1	5	0	0	0
Water Compatible	98	8	10	2	5	0	0	0
Total number with decrease (excluding water compatible)								265
Total number with increase (excluding water compatible)								73



## **B6. 1% AEP event (100-year return period event)**

A comparison between baseline and Mitigated Scheme scenarios for the original model indicates that 644 receptors are predicted to decrease in flood depth, whilst 306 receptors are predicted to increase.

Predicted increases smaller than 0.01m (10mm) and therefore considered “negligible” impacts are summarised as follows:

- One “Essential Infrastructure” receptor. This is a sewage pumping station near Fosse Road with an increase of 0.004m (4mm) on top of a baseline flood depth of 0.6m (600mm). Flood hazard is not predicted to change.
- Five “Highly Vulnerable” receptors with increases up to 0.002m (2mm). Two of these are caravans at Tolney Lane, with increases of 2mm on top of baseline depths of 0.1m (100mm), the remaining three are telecommunications phone masts with increases of less than 0.002m (2mm) on top of baseline depths of 0.27m (270mm).

The model predicts flood depth increases above 0.01m (10mm), however investigation of model results indicates that changes in flood level at their locations are sensitive to model configuration. This is discussed further in Sections B6.1 and B6.2 of this Appendix. The receptors with predicted increases above 0.01m (10mm) are summarised as follows:

- One “Essential Infrastructure” receptor. This is an electricity sub-station near Fosse Road with an increase of 0.027m (27mm) on top of a baseline flood depth of 0.22m (220mm). Flood hazard is not predicted to change.
- 36 “More Vulnerable” receptors, all of which are located near Fosse Road, 300m southwest of Farndon Roundabout, and are attributed to modelling uncertainties. Impacts at Fosse Road are discussed further in sections B6.1 and B6.2 of this Appendix. It should be noted that except for two receptors, flood hazard is not predicted to change; the two “More Vulnerable” residential properties show an increase in flood hazard classification from “Low” to “Moderate”. However, this is reflective only of the NRD point position at the centroid of the properties. The overall hazard across the wider area is “Moderate”.
- Two of the 36 “More Vulnerable” receptors near Fosse Road are not predicted to flood in the baseline and are now predicted to flood due to the Scheme, these are dwellings at Village Close. The hazard classification at these receptors with the Scheme is “Low”. The flood depth with the Scheme is up to 0.021m (21mm) at these properties.
- Five “More Vulnerable (Low Confidence)” receptors located near Fosse Road 300m southwest of Farndon Roundabout. Flood hazard is not predicted to change at these receptors.
- Three “Less Vulnerable” receptors, two of which are the Farndon West and East FCAs, and the third is a showroom at Fosse Road. At the showroom, a depth increase of 0.027m (27mm) is predicted on top of a baseline depth of 0.310m (310mm), and the flood hazard is not predicted to change.



## B6.1 1% AEP receptors at Fosse Road

In the vicinity of Fosse Road, flood depth increases greater than 0.01m (10mm) are observed in the original model (Figure 16). One “Essential Infrastructure” receptor, and all “More Vulnerable” and “More Vulnerable (Low Confidence)” receptors with an increase above 0.01m (10mm) in the 1% AEP event are located in this area.

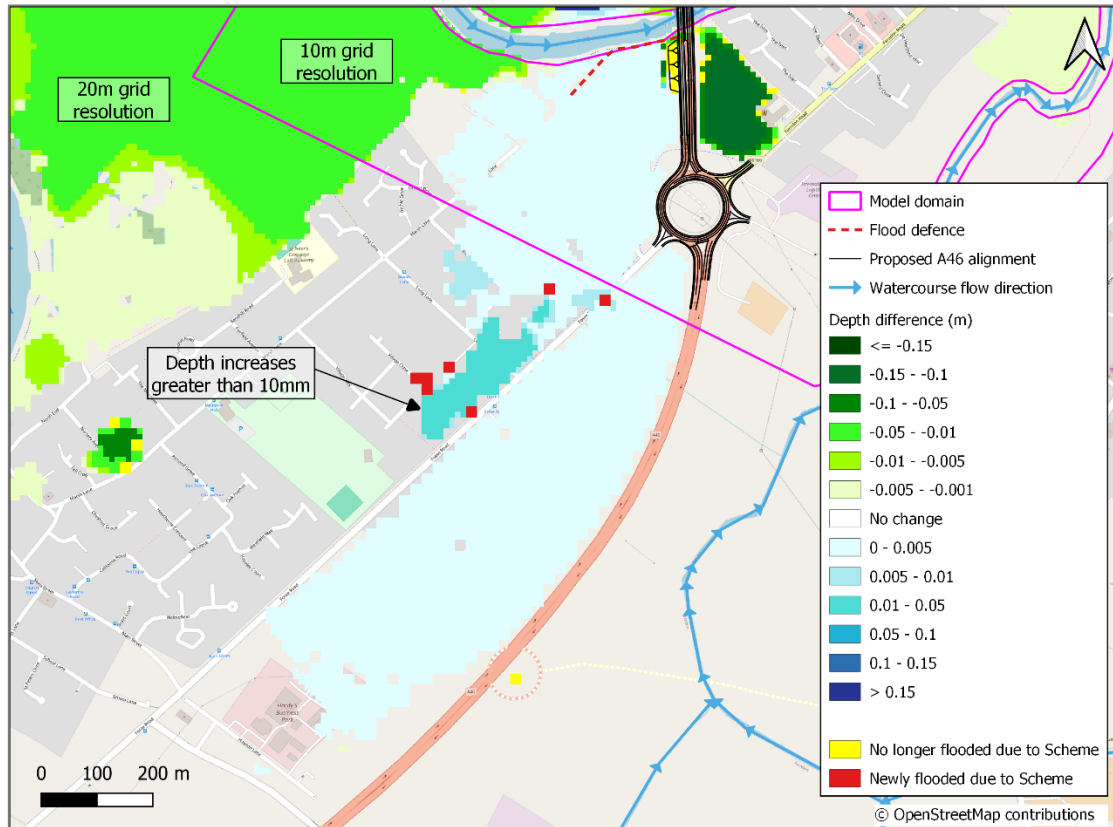


Figure 16: 1% AEP. Fosse Road. Flood depth differences. Mitigated Scheme versus baseline. Original model.

A review of model grid cells at this location indicates that this impact can be attributed to modelling uncertainties arising from the selection of grid cell size(s), as discussed in Section 2.2. Figure 16 shows the transition from a 10m grid resolution, representing the Scheme, to a 20m grid resolution beyond the Scheme boundary. The area of flood depth increase at Fosse Road is outside of the Scheme boundary, and the grid size transition occurs approximately 150m north of this area, resulting in edge effects at the model domain boundary.

Sensitivity testing of the model domain boundary locations has been undertaken to further investigate this area of flood depth increase and is discussed in section B6.2 of this Appendix.

## B6.2 Sensitivity testing

As the area of flood depth increase greater than 0.01m (10mm) in the vicinity of Fosse Road is limited to the 1% AEP event only, sensitivity testing was undertaken for this event. The sensitivity test combined the modified representation of the Scheme embankment and abutment at Windmill Viaduct as discussed in Section 3.2, with modifications to the model domain boundaries.

The original model domain boundary and the model domain boundary modified as part of the sensitivity test are shown in Figure 17. This boundary marks the change in grid resolution from 10m to 20m. For the sensitivity test, the ground model in the Fosse Road area is represented at a higher (10m) resolution.

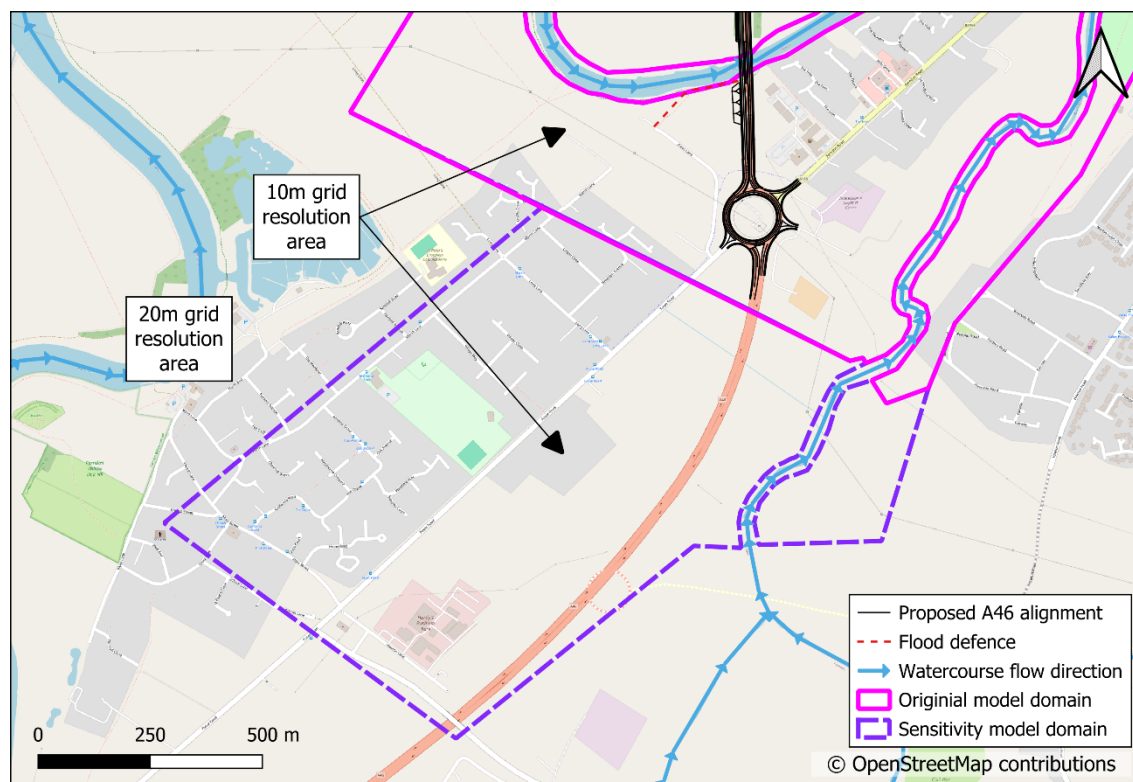


Figure 17: Original 10m model domain (pink solid line) and its extension further south in the sensitivity model (purple dashed line)

Flood depth changes near Fosse Road following the sensitivity test are shown in Figure 18. The sensitivity model does not predict any increases in flood depth greater than 0.01m (10mm) at any “Essential Infrastructure”, “Highly Vulnerable”, “More Vulnerable”, “More Vulnerable (Low Confidence)”, or “Less Vulnerable (Low Confidence)” receptors. The only predicted increases above 0.01m (10mm) are at the “Less Vulnerable” FCAs as expected.

There are three instances of new flooding, shown by red cells in Figure 18, with flood depths ranging between 0.001m to 0.02m (1mm to 20mm). These are located at the edges of the Farndon Recreation Ground and do not impact any vulnerable receptors.

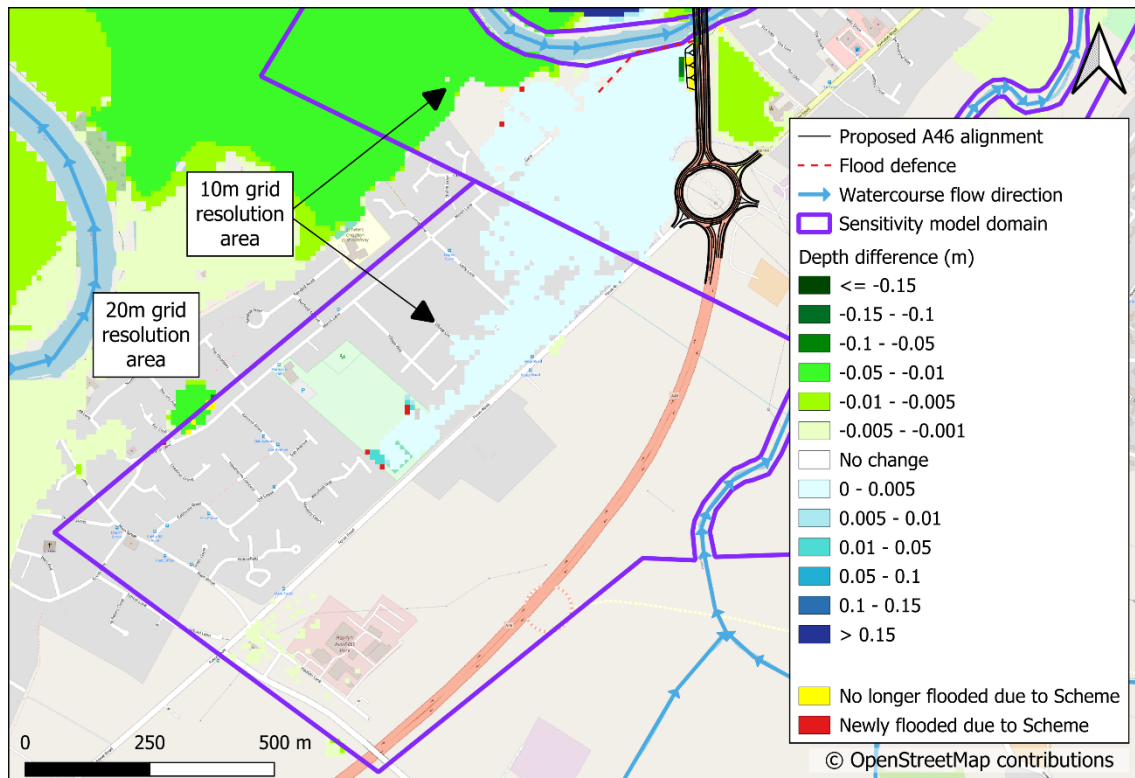


Figure 18: 1% AEP. Fosse Road. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.

### B6.3 Summary – 1% AEP

The original model predicts that flood depths would increase by more than 0.01m (10mm) at one “Essential Infrastructure”, 36 “More Vulnerable”, five “More Vulnerable (Low Confidence)” and three “Less Vulnerable” receptors in the Fosse Road area. Flood hazard is not predicted to increase at these receptors.

Sensitivity testing was undertaken to determine if these increases were sensitive to model inputs near Fosse Road. For the sensitivity test, modifications were made to Scheme embankment and abutment at Windmill Viaduct, together with adjustments to the location of the model domain boundary. As a result, there are no flood depth increases greater than 0.01m (10mm), aside from at two “Less Vulnerable” receptors at Farndon West and East FCAs, as expected.

Figure 10 presents the receptor analysis for the 5% AEP event for the original model, and Figure 11 presents the receptor analysis incorporating the sensitivity test.

Table 10: 1% AEP. Flood depth differences. Mitigated Scheme versus baseline. Original model.

	Count of receptors with change in depth							
NPPF Class	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	9	0	0	0	1	0	0	1
Highly Vulnerable	63	2	3	0	0	0	0	0
More Vulnerable	220	14	20	9	49	40	17	36
More Vulnerable (Low Confidence)	106	18	10	6	4	19	4	5
Less Vulnerable	166	4	5	1	3	9	0	3
Less Vulnerable (Low Confidence)	80	5	10	1	2	5	0	0
Water Compatible	141	7	7	2	0	0	0	3
Total number with decrease (excluding water compatible)								644
Total number with increase (excluding water compatible)								306

Table 11: 1% AEP. Flood depth differences. Mitigated Scheme versus baseline. Sensitivity model.

	Count of receptors with change in depth							
NPPF Class	<0mm	0-1mm	1-2mm	2-3mm	3-4mm	4-5mm	5-10mm	>10mm
Essential Infrastructure	4	0	1	0	0	0	0	0
Highly Vulnerable	61	1	0	0	0	0	0	0
More Vulnerable	174	57	101	4	1	0	0	0
More Vulnerable (Low Confidence)	66	31	18	1	0	0	0	0
Less Vulnerable	123	7	10	1	0	0	0	2
Less Vulnerable (Low Confidence)	44	8	13	1	0	0	0	0
Water Compatible	100	8	8	1	0	0	0	0
Total number with decrease (excluding water compatible)								472
Total number with increase (excluding water compatible)								257